

Bailey's Production Facility, Mallusk. Tegral Twin-Skin Roofing and Cladding



Independent News & Media plc. Tegral Composite Floor Decking

Central Park, Leopardstown, Dublin. Tegral Composite Floor Decking

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Tegral Roof Decking

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A world of experience

Our Company

Tegral Metal Forming is part of the Tegral Group and a subsidiary of the Etex Group, a world-renowned international building products company. For over 25 years, Tegral Metal Forming has been to the forefront of development with regard to roofing, cladding and flooring systems.

Based in Athy, Co. Kildare, the Tegral Group consists of Tegral Building Products and Tegral Metal Forming. Tegral Building Products is Ireland's largest manufacturer and distributor of roofing products and Tegral Metal Forming Ltd. is a leading manufacturer and supplier of metal roofing, cladding and flooring systems for the construction industry.

The comprehensive product range is designed to suit most applications in modern commercial, industrial and agricultural construction. Over the years, Tegral Metal Forming has developed an expertise in every aspect of metal systems application.

Project: Colaiste de hIdé, Dublin Architects: Campbell Conroy Hickey Product: Tegral Fineline 19, Corus Colorcoat[®] HPS200

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Our Partners

Through a long-standing partnership with Corus, a world-renowned manufacturer of steel and aluminium, Tegral customers and specifiers are assured of the highest standards and quality in all Tegral products.

Our Standards

All manufacturing in Athy meets with the stringent requirements of Quality Assurance systems to ISO EN 9001:2000.

Our People

People really do matter at Tegral Metal Forming. Recently the company proudly embraced and succeeded in achieving the

"Excellence Through People" award, Ireland's national standard for human resource development.

Our Industry Associates

Tegral Metal Forming takes an active role in the promotion of the metal industry and is involved in the Roof Manufacturers and Suppliers Association (RMSA) in Ireland, the Metal Cladding and Roofing Manufacturers Association (MCRMA) in the UK and also the Irish Farm Buildings Association.

Tegral Purlins and Rails

'Good designers appreciate the finest materials'.

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When it comes to purlins and rails, the Tegral Zeta range is clearly a brand leader in Ireland today. For over 20 years Tegral has supplied the island of Ireland from its central location in Co Kildare. The production of cold-rolled steel profiles requires specialist skills, superior manufacturing capabilities and top quality materials. Due to the complex nature of working with purlins, professional technical support service are important.

Tegral has taken the production of roll-forming metal to new heights. Over the years Tegral has developed and improved the product range to such an extent that Tegral Purlins and Rails have now set the industry standard for the quality of steel used. Using "S390" means greater working capacities for Tegral purlins, rails and complementary sections.

The Tegral Technical Services Department has dedicated purlins and rails professionals who are available to assist and support on all projects.

Purlins Rails

Computer Aided Design

A selection of software is available upon request from the Tegral's Technical Services team to facilitate the design and detailing of Tegral Zeta purlin and rail systems.

Ordering Purlins and Rails

Ordering purlins and rails from Tegral Metal Forming is now easier than every before thanks to a new free step by step guide called ' How to Order Purlins using Strucad[©] Software'. Contact Technical Services for further details.

Tegral product range

Tegral product range

LPCB & FM approved Insulated Panel Range

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ComFlor® Flooring Range

Flat Roof Deck Range

Zeta Purlin Range

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DESIGNER'S PURLIN & RAIL GUIDE

oduct Range

Purlins Rails - Product Range

The following information is provided as a general guide to the purlin and cladding rail systems available from Tegral Metal Forming. It is

It is intended to offer indicative guidance to the range of sections, systems and ancillary members produced, and their proposed applications.

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Systems

Sleeved

Single spanning sections with sleeves at alternate supports. Sleeves act as 'mechanical hinges' to provide maximum structural efficiency for the number of components employed. Single span purlins provide manageable lengths on site.

Suitable for most building applications.

Heavy End Bay

Single spanning sections with sleeves at all supports providing a fully continuous system. End bay sections and sleeves increased in gauge to compensate for the lack of continuity at the gable. High load carrying capacity for larger spans and/or heavier loadings.

Suitable as a purlin system for buildings with a large number of bays (say 10 No.).

Butted

Single spanning sections without sleeves. Non continuous system.

Suitable where depth constraints exist and sections are fixed between supports.

Double Span

Double spanning sections with only a minimum number of sleeves to alternate purlins across the penultimate support. Does not give quite the same strength capacity as the Sleeved system, but uses fewer components and is therefore faster to erect.

Alternative to the Sleeved system for spans up to 7.50m.

Double Span Heavy End Bay

Similar to Heavy End Bay system, but using double spanning interior purlin lengths and fewer sleeves. Does not give quite the same strength capacity as the Heavy End Bay system, but uses fewer components and is therefore faster to erect.

Alternative to the Heavy End Bay system for spans up to 7.50m.

Complimentary sections

Eaves Beam

Specially designed to combine the function of purlin, cladding rail and gutter support at the eaves position. Depths available: 160 and 240mm

Panel Joint Rail and Top Hat Sections

Developed for the support of horizontal cladding panels. Intended to span vertically between conventional horizontal rails (Panel Joint Rails only). Depths available: PJR: 140 - 200mm TH: 80 and 100mm

'C' & 'CW' Sections

A variety of sections available for use as: Cladding rails, Masonry wall restraints, Door/window restraints, Parapet capping rails. Depths available: C: 90 - 160mm, CW: 127 - 240mm

Sleeve detail.

brace position.

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Typical Arrangements

DETAIL 3 Corner column cleat detail using extended cleat.

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Rail and cleat assembly at diagonal

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Zeta Section Properties

No		ss t	ige a	nge b		b	e punch	ed in any	y pattern	on thes	e standa	ard lines.
Section	Depth d	Thickne	Top Flar	Btm Fla	Weight	Area	Ixx	ZXX	lyy	ſŊ	×	*
	mm	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	mm
* 1/12513	125	1.3	60	50	2.73	348	83.6	12.84	11.85	18.81	21.39	65.10
* 1/12514	125	1.4	60	50	2.95	375	90.0	13.82	12.78	18.79	21.45	65.11
* 1/12515	125	1.5	60	50	3.16	402	96.4	14.81	13.71	18.78	21.51	65.12
* 1/12517	125	1.7	60	50	3.58	456	109.2	16.76	15.55	18.75	21.62	65.15
* 1/12520	125	2.0	60	50	4.21	536	128.0	19.64	18.29	18.71	21.80	65.18
1/15013	150	1.3	72	65	3.28	417	147.4	19.04	22.27	23.56	21.39	77.42
1/15014	150	1.4	72	65	3.53	449	158.8	20.51	24.02	23.54	21.45	77.43
1/15015	150	1.5	72	65	3.78	482	170.2	21.98	25.77	23.53	21.51	77.44
1/15017	150	1.7	72	65	4.28	546	192.9	24.90	29.25	23.50	21.63	77.47
1/15020	150	2.0	72	65	5.04	642	226.5	29.22	34.43	23.45	21.81	77.50
1/17513	175	1.3	72	65	3.53	450	211.2	23.43	22.28	22.70	21.28	90.14
1/17514	175	1.4	72	65	3.80	484	227.7	25.25	24.03	22.68	21.34	90.15
1/17515	175	1.5	72	65	4.07	519	244.0	27.07	25.78	22.67	21.39	90.16
1/17517	175	1.7	72	65	4.62	588	276.6	30.67	29.26	22.64	21.51	90.19
1/17520	175	2.0	72	65	5.43	692	325.0	36.02	34.45	22.60	21.69	90.23
1/20013	200	1.3	72	65	3.79	482	288.9	28.10	22.29	21.92	21.18	102.83
1/20014	200	1.4	72	65	4.08	519	311.5	30.29	24.04	21.91	21.24	102.84
1/20015	200	1.5	72	65	4.37	557	334.0	32.47	25.78	21.89	21.30	102.86
1/20016	200	1.6	72	65	4.66	594	356.4	34.64	27.53	21.88	21.36	102.87
1/20018	200	1.8	72	65	5.24	668	400.9	38.96	31.00	21.85	21.47	102.90
1/20020	200	2.0	72	65	5.82	742	445.2	43.25	34.46	21.82	21.59	102.93
1/20025	200	2.5	72	65	7.22	920	554.4	53.83	43.01	21.84	21.88	102.99

Zeta Purlins

Standard holing lines for all sections 18 dia. or 14 sq. web holes, and 14 dia. flange holes may

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support of alternate purlin lines to maintain

continuity (see page 5 for alternative joint

system, but requires fewer components.

The double span system does not quite achieve the

same strength capacity as the equivalent sleeved

arrangement diagram).

This system utilises single bay purlin lengths joined with connecting sleeves at alternate supports. All purlins must be sleeved across the penultimate support (see page 4 for alternative joint arrangement diagram). This system is considered to be the most efficient on the

	E	4.	.5	5	.0	5	.5	6	5.0		6.5	7	.0	7.	5	8.	0	8	.5
No.	: kg/r	metre	span	metr	e span	metre	e span	metre	e span	metro	e span	metr	e span	metr	e span	metre	e span	metre	span
Section	Self wt	Gravity	Uplift																
1/12513	2.73	7.684	7.926	5.870	6.138	5.071	5.366	3.917	4.239										
1/12514	2.95	8.272	8.532	6.319	6.608	5.459	5.777	4.217	4.563										
1/12515	3.16	8.861	9.139	6.769	7.078	5.847	6.187	4.517	4.888										
1/15013	3.28	11.553	11.522	10.367	10.400	9.394	9.486	7.921	8.307	6.206	6.624								
1/15014	3.53	12.488	12.799	11.206	11.552	10.154	10.535	8.534	8.949	6.687	7.136								
1/15015	3.78	13.702	14.036	12.296	12.667	10.946	11.354	9.146	9.591	7.167	7.649								
1/17513	3.53	13.608	13.212	12.215	11.924	11.071	10.873	10.115	10.000	8.599	9.049	7.292	7.777	6.163	6.683	5.172	5.727		
1/17514	3.80	14.754	14.323	13.243	12.926	12.004	11.786	10.942	10.840	9.271	9.756	7.862	8.384	6.645	7.204	5.577	6.174		
1/17515	4.07	16.227	16.587	14.567	14.966	13.204	13.644	11.725	12.204	9.935	10.455	8.425	8.984	7.120	7.720	5.976	6.616		
1/20013	3.79	15.514	15.042	13.927	13.573	12.626	12.375	11.538	11.379	10.615	10.539	9.821	9.822	9.130	9.203	8.340	8.664	6.804	7.436
1/20014	4.08	16.887	16.369	15.160	14.770	13.744	13.466	12.560	12.382	11.556	11.468	10.692	10.687	9.940	10.013	8.993	9.426	7.337	8.017
1/20015	4.37	18.632	19.018	16.728	17.157	15.167	15.638	13.862	14.376	12.754	13.311	11.802	12.402	10.974	11.616	9.642	10.328	7.867	8.596
1/20016	4.66	20.354	20.765	18.275	18.732	16.570	17.073	15.145	15.694	13.936	14.531	12.897	13.537	11.780	12.466	10.289	11.021	8.395	9.172
1/20018	5.24	23.740	24.203	21.317	21.831	19.330	19.896	17.670	18.287	16.261	16.930	15.050	15.770	13.251	14.022	11.574	12.397	9.443	10.318

General notes to all Zeta purlin systems

Working load capacities for gravity loading are printed on a blue background. These values are derived from the Ultimate Load Capacities given on pages 74-75 divided by 1.6. Working load capacities for wind uplift are printed on a grey background. These values are derived from the Ultimate Load Capacities given on pages 74-75 divided by 1.4.

Zeta Double Span Purlins

This system has been developed to utilise a minimum number of components leading to maximum speed of erection on site. The purlins are continuous over two bays with simple connections at each support. Joints are staggered with a minimum number of sleeves being required across the penultimate

Working Load Capacities in kN

	E.	4	1.5	5	.0	5.	.5	é	5.0	6	.5	7	.0	7.	5
n No.	/t kg/	metr	e span	metre	e span	metre	span	metr	e span	metre	span	metre	e span	metre	span
Sectio	Self w	Gravity	Uplift												
1/12513	2.73	7.790	8.032	6.390	6.659	5.373	5.669	4.325	4.646						
1/12514	2.95	8.530	8.790	6.880	7.169	5.785	6.103	4.656	5.002						
1/12515	3.16	9.260	9.539	7.369	7.678	6.196	6.537	4.987	5.358						
1/15013	3.28	10.578	10.868	9.490	9.812	8.597	8.951	7.849	8.236	6.674	7.092				
1/15014	3.53	11.765	12.076	10.556	10.902	9.563	9.944	8.733	9.148	7.190	7.640				
1/15015	3.78	12.909	13.243	11.583	11.954	10.495	10.902	9.443	9.887	7.707	8.189				
1/17513	3.53	12.461	12.773	11.182	11.529	10.133	10.514	9.255	9.671	8.510	8.960	7.869	8.354	6.907	7.427
1/17514	3.80	13.901	14.237	12.476	12.849	11.306	11.716	10.328	10.776	9.498	9.983	8.696	9.218	7.447	8.007
1/17515	4.07	15.289	15.649	13.722	14.122	12.437	12.876	11.362	11.842	10.450	10.969	9.319	9.878	7.980	8.580
1/20013	3.79	14.207	14.542	12.751	13.123	11.557	11.965	10.558	11.004	9.710	10.193	8.981	9.501	8.346	8.903
1/20014	4.08	15.911	16.271	14.282	14.682	12.945	13.385	11.828	12.308	10.880	11.400	10.064	10.624	9.355	9.955
1/20015	4.37	17.556	17.942	15.760	16.188	14.286	14.757	13.054	13.569	12.009	12.566	11.110	11.710	10.328	10.971
1/20016	4.66	19.179	19.590	17.217	17.674	15.608	16.111	14.264	14.812	13.123	13.717	12.141	12.781	11.288	11.973
1/20018	5.24	22.369	22.832	20.083	20.598	18.209	18.774	16.642	17.259	15.312	15.981	14.169	14.889	13.175	13.946

General notes to all Zeta purlin systems

Working load capacities for gravity loading are printed on a blue background. These values are derived from the Ultimate Load Capacities given on pages 74-75 divided by 1.6. Working load capacities for wind uplift are printed on a grey background. These values are derived from the Ultimate Load Capacities given on pages 74-75 divided by 1.4.

The effect of purlin self weight is included in the tabulated loads above. The tabulated values will give deflections not exceeding span/180.

The load capacities may be achieved without the use of sag bars for most conditions where a steel sheeting system is screw fixed to the purlins (see page 14 for further information and limitations). For applications where any other type of sheeting system is employed, i.e. other than screw fixed steel sheeting, please consult our Technical Services Department.

Zeta Sleeved Purlins

majority of buildings for bay centres up to 7.5m approx. The use of single span purlin lengths ensures easier on site handling and compliance with Health & Safety Authority requirements.

Working Load Capacities in kN

The effect of purlin self weight is included in the tabulated loads above. The tabulated values will give deflections not exceeding span/180.

The load capacities may be achieved without the use of sag bars for most conditions where a steel sheeting system is screw fixed to the purlins (see page 14 for further information and limitations). For applications where any other type of sheeting system is employed, i.e. other than screw fixed steel sheeting, please consult our Technical Services Department.

The Double Span System comprises a two bay spanning purlin with simple connections at supports.

Sleeved and Heavy End Bay Systems

The Sleeved System comprises a single bay purlin with a sleeve connection at alternate supports.

Note: Sleeves on every joint at penultimate rafter.

General Detailing Notes

All purlins must be fixed with the top flange facing the apex. See page 14 for minimum sag bar requirements.

All fixing holes are 18 dia. for M16 grade 4.6 bolts. Sag Bar/Bracing Strut holes are 14 sq. punched in pairs.

Sheeting line depth using standard Zeta cleats equals section depth +7mm.

This system is usually more advantageous for longer

spans and/or heavier loadings where there are a

compared to the equivalent sleeved system, with

minimum of six bays. Providing sleeves at all

The effect of purlin self weight is included in the tabulated

loads above. The tabulated values will give deflections not

The load capacities may be achieved without the use of sag bars for

most conditions where a steel sheeting system is screw fixed to the

purlins (see page 14 for further information and limitations). For

i.e. other than screw fixed steel sheeting, please consult our

applications where any other type of sheeting system is employed,

exceeding span/180.

Technical Services Department.

supports increases the capacity by over 25%

sleeves at alternate joints.

This is a fully continuous system with sleeves provided at all supports. The end bay purlins and sleeves to the penultimate support are increased in gauge to compensate for the lack of continuity across the gable, hence the name (see page 5 for alternative joint arrangement diagram).

Working Load Capacities in kN

	ction	4	.5	5	.0	5	.5	6.	0	6	.5	7.	.0	7.	5	8.	D	8.	5
n No.	ay Se	metre	e span	metre	e span	metre	e span	metre	span	metre	e span	metre	span	metre	span	metre	span	metre	span
Interio	End Ba	Gravity	Uplift																
1/15013	.17	14.300	14.194	12.840	12.805	11.642	11.672	10.366	10.730	8.122	8.669								
1/15014	.20	15.903	16.214	14.279	14.626	12.948	13.329	11.836	12.251	9.537	10.179								
1/15015	.20	17.448	17.781	15.668	16.038	14.208	14.616	12.171	12.765	9.537	10.179								
1/17513	.17	16.841	16.278	15.124	14.683	13.716	13.382	12.540	12.300	11.262	11.387	9.551	10.185						
1/17514	.20	18.784	18.145	16.870	16.366	15.301	14.914	13.990	13.707	12.878	12.688	11.221	11.818						
1/17515	.20	20.659	21.018	18.555	18.954	16.830	17.270	15.389	15.869	13.233	13.925	11.221	11.967						
1/20013	.18	19.197	18.535	17.242	16.717	15.639	15.232	14.300	13.999	13.164	12.958	12.188	12.068	11.340	11.299	10.595	10.629		
1/20014	.18	21.496	20.741	19.309	18.705	17.515	17.043	16.017	15.661	14.747	14.495	13.655	13.498	12.706	12.637	11.873	11.886		
1/20014	.20	21.496	20.741	19.309	18.705	17.515	17.043	16.017	15.661	14.747	14.495	13.655	13.498	12.706	12.637	11.873	11.886		
1/20015	.20	23.716	24.102	21.304	21.733	19.326	19.798	17.675	18.189	16.274	16.831	15.070	15.670	14.024	14.667	12.853	13.767	10.487	11.458
1/20016	.25	25.168	25.580	22.608	23.065	20.509	21.012	18.756	19.305	17.270	17.864	15.992	16.632	14.882	15.567	13.907	14.639	13.045	13.822
1/20018	.25	30.160	30.623	27.095	27.610	24.583	25.149	22.485	23.102	20.706	21.375	19.178	19.898	17.849	18.621	16.007	17.141	13.061	14.266

General notes to all Zeta purlin systems

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DESIGNER'S PURLIN & RAIL GUIDE

Zeta Purlin Detailing

Double Span System

Note: End bays that require a single bay purlin will need a sleeve connection at the penultimate rafter.

The Heavy End Bay System comprises a single bay purlin with a sleeve connection at every support.

Note: End bays require the appropriate heavier gauge section and sleeve at the penultimate rafter.

STANDARD	DIM	ENSION	S FOR Z	ETA SEC	TIONS
Purlin depth	d	125	150	175	200
Flange widths	а	60	72	72	72
	b	50	65	65	65
Hole centres	С	62	74	87	64
	е	0	0	0	70
	f	63	76	88	66
Sleeve	g	250	315	390	405
Dimensions	h	695	825	975	1000
					001

DESIGNER'S PURLIN & RAIL GUIDE

Zeta Purlin Accessories

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Zeta Purlin Accessories

Sag Bar Requirements

Where adequate lateral restraints is afforded by steel sheeting screw fixed to the purlin flange, sag bars are not required to develop the load capacities given on pages 10-12.

To maintain the correct alignment and to help prevent distortion of the purlin during sheeting, sag bars should be fitted in accordance with our minimum recommendations shown in the table below.

PU	RLIN SP/	AN IN METRES	
Roof pitch θ	S ≤ 4.5	4.5 < S ≤ 7.6	S >7.6
$0^{\circ} < \theta \le 3^{\circ}$	Consult	Technical Service	es Dept.
3° < θ ≤12°	0	0*	2*
12° < θ ≤ 20°	0	1*	2*
θ > 20°	Consult	Technical Service	es Dept.

For purlin in excess of 2m, Heavy Duty Sag Bars should be used as required.

*See section headed "Eaves Tie and Fixing Bracket" for guidance on restraint to first purlins at eaves.

For roof slope lengths in excess of 20m, it may become necessary to introduce additional support along the roof slope in the form of diagonal brace section tied back to the rafters. Details of the diagonal braces are shown on page 21. Please consult the Technical Services Department for further advice.

Special consideration must be given to purlins clad with any material other than screw fixed profiled steel sheeting. These include fibre cement sheeting, standing seam clip fix profiles and tiled roof constructions.

Details required from customer: Now availabl Dimension A (Note: measured to heel of flange) Dimension H Patrick Lynch Roof Cladding info@patricklynchroofcladding.com www.patricklynchroofcladding.com

Eaves tie and fixing bracket

To ensure the first purlin at eaves is kept straight, particularly during the early stages of sheeting, it is necessary to give consideration to additional restraint to this purlin, especially on spans of 5m or more. This may be done by providing an eaves tie of the type shown, or in agreement with the sheeting contractor, a temporary prop may be used.

Rafter Stays

The same section used to form the eaves tie is also available as a rafter stav member, where these are specified by the frame designer. A special cranked version with a 20mm offset is used for Zeta purlins.

Cleader Members

50 x 50 x 1.5mm thick galvanised steel Cleader Angle, avaliable in 4m unpunched lengths.

100 x 100 x 1.5mm thick galvanised steel verge trim angle for fixing to the top flange, available in 4m unpunched lenghts.

17

Zeta Section Properties

Section No	Depth d	Thickness t	Top Flange a	Btm Flange b	Weight	Area	XXI	ZXX	۲ <mark>۸</mark>	ryy	×	>
	mm	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	mm
*1/12513	125	1.3	60	50	2.73	348	83.6	12.84	11.85	18.81	21.39	65.10
*1/12514	125	1.4	60	50	2.95	375	90.0	13.82	12.78	18.79	21.45	65.11
*1/12515	125	1.5	60	50	3.16	402	96.4	14.81	13.71	18.78	21.51	65.12
*1/12517	125	1.7	60	50	3.58	456	109.2	16.76	15.55	18.75	21.62	65.15
*1/12520	125	2.0	60	50	4.21	536	128.0	19.64	18.29	18.71	21.80	65.18
1/15013	150	1.3	72	65	3.28	417	147.4	19.04	22.27	23.56	21.39	77.42
1/15014	150	1.4	72	65	3.53	449	158.8	20.51	24.02	23.54	21.45	77.43
1/15015	150	1.5	72	65	3.78	482	170.2	21.98	25.77	23.53	21.51	77.44
1/15017	150	1.7	72	65	4.28	546	192.9	24.90	29.25	23.50	21.63	77.47
1/15020	150	2.0	72	65	5.04	642	226.5	29.22	34.43	23.45	21.81	77.50
1/17513	175	1.3	72	65	3.53	450	211.2	23.43	22.28	22.70	21.28	90.14
1/17514	175	1.4	72	65	3.80	484	227.7	25.25	24.03	22.68	21.34	90.15
1/17515	175	1.5	72	65	4.07	519	244.0	27.07	25.78	22.67	21.39	90.16
1/17517	175	1.7	72	65	4.62	588	276.6	30.67	29.26	22.64	21.51	90.19
1/17520	175	2.0	72	65	5.43	692	325.0	36.02	34.45	22.60	21.69	90.23
1/20013	200	1.3	72	65	3.79	482	288.9	28.10	22.29	21.92	21.18	102.83
1/20014	200	1.4	72	65	4.08	519	311.5	30.29	24.04	21.91	21.24	102.84
1/20015	200	1.5	72	65	4.37	557	334.0	32.47	25.78	21.89	21.30	102.86
1/20016	200	1.6	72	65	4.66	594	356.4	34.64	27.53	21.88	21.36	102.87
1/20018	200	1.8	72	65	5.24	668	400.9	38.96	31.00	21.85	21.47	102.90
1/20020	200	2.0	72	65	5.82	742	445.2	43.25	34.46	21.82	21.59	102.93
1/20025	200	2.5	72	65	7.22	920	554.4	53.83	43.01	21.84	21.88	102.99

Zeta Rails

*By Special order

Standard holing lines for all sections

18 dia. or 14 sq. web holes, and 14 dia. flange holes may be punched in any pattern on these standard lines.

This system utilises single bay rail lengths joined with connecting sleeves at alternate supports.

	Ę	4	.5	5	.0	5	.5	6	.0	6.	5	7	.0	7.	.5	8.	0	8.	5
NO.	vt kg/	metro	e span	metre	span	metre	e span	metre	e span	metre	span	metre	e span	metre	e span	metre	span	metre	span
Sectio	Self v	Pressure	Suction																
1/12513	2.73	9.366	8.393	7.205	7.205	6.262	6.262	4.894	4.894	4.294	4.294								
1/12514	2.95	10.083	9.185	7.756	7.756	6.741	6.741	5.268	5.268	4.623	4.623								
1/12515	3.16	10.800	9.969	8.308	8.308	7.221	7.221	5.643	5.643	4.952	4.952								
1/15013	3.28	13.369	11.377	12.032	10.240	10.939	9.309	9.737	8.533	7.698	7.698	5.965	5.965						
1/15014	3.53	14.450	12.644	13.005	11.379	11.823	10.345	10.490	9.483	8.294	8.294	6.426	6.426						
1/15015	3.78	15.850	13.869	14.265	12.482	12.968	11.347	11.243	10.402	8.889	8.889	6.888	6.888						
1/17513	3.53	15.731	13.056	14.158	11.751	12.870	10.682	11.798	9.792	10.589	9.039	9.042	8.393	7.707	7.707	6.540	6.540		
1/17514	3.80	17.054	14.155	15.348	12.739	13.953	11.581	12.790	10.616	11.416	9.799	9.748	9.099	8.309	8.309	7.050	7.050		
1/17515	4.07	18.751	16.407	16.876	14.767	15.342	13.424	14.063	12.305	12.234	11.359	10.446	10.446	8.904	8.904	7.555	7.555		
1/20013	3.79	17.921	14.875	16.129	13.387	14.663	12.170	13.441	11.156	12.407	10.298	11.521	9.562	10.753	8.925	10.081	8.367	8.544	7.875
1/20014	4.08	19.505	16.189	17.554	14.570	15.959	13.246	14.629	12.142	13.503	11.208	12.539	10.407	11.703	9.713	10.972	9.106	9.213	8.571
1/20015	4.37	21.514	18.825	19.363	16.942	17.603	15.402	16.136	14.119	14.894	13.033	13.831	12.102	12.909	11.295	11.982	10.589	9.878	9.878
1/20016	4.66	23.497	20.560	21.147	18.504	19.225	16.822	17.623	15.420	16.267	14.234	15.105	13.217	14.098	12.336	12.786	11.565	10.540	10.540
1/20018	5.24	27.396	23.971	24.656	21.574	22.415	19.613	20.547	17.978	18.966	16.595	17.611	15.410	16.364	14.383	14.382	13.484	11.857	11.857

General notes to all Zeta cladding rail systems

Working load capacities for wind pressure (wide, sheeted flange in compression at mid span) are printed on a blue background. These values are derived from the Ultimate Load Capacities given on page 80 divided by 1.4. Working load capacities for wind suction (narrow, unsheeted flange in compression at mid span) are printed on a grey background.

Zeta Double Span Rails

This system utilises a minimum number of components leading to maximum speed of erection on site. The rails are continuous over two bays with simple connections at the each support.

General notes to all Zeta cladding rail systems

Working load capacities for wind pressure (wide, sheeted

flange in compression at mid span) are printed on a blue background. These values are derived from the

flange in compression at mid span) are printed on a grey

Ultimate Load Capacities given on pages 74-75 divided by 1.4.

Working load capacities for wind suction (narrow, unsheeted

Joints are staggered with a minimum number of sleeves being required across the penultimate support of alternate rail lines to maintain continuity (see page 5 for alternative joint arrangement diagram).

Working Load Capacities in kN

	F	4	.5	5	.0	5	5.5	6	.0	6.	.5	7	.0	7	.5
No.	t kg/r	metre	e span	metre	e span	metr	e span	metre	e span	metre	span	metre	e span	metre	e span
Sectior	Self wi	Pressure	Suction												
1/12513	2.73	9.041	7.911	7.830	7.120	6.625	6.473	5.383	5.383	4.649	4.649				
1/12514	2.95	9.897	8.660	8.429	7.794	7.132	7.085	5.795	5.795	5.005	5.005				
1/12515	3.16	10.742	9.400	9.028	8.460	7.640	7.640	6.207	6.207	5.361	5.361				
1/15013	3.28	12.255	10.723	11.030	9.651	10.027	8.774	9.191	8.042	8.260	7.424	6.785	6.785	6.428	6.428
1/15014	3.53	13.624	11.921	12.261	10.729	11.147	9.753	10.218	8.941	8.898	8.253	7.310	7.310	6.926	6.926
1/15015	3.78	14.944	13.076	13.449	11.768	12.227	10.698	11.208	9.807	9.537	9.053	7.835	7.835	7.423	7.423
1/17513	3.53	14.420	12.617	12.978	11.355	11.798	10.323	10.815	9.463	9.983	8.735	9.270	8.111	8.600	7.570
1/17514	3.80	16.079	14.069	14.471	12.662	13.155	11.511	12.059	10.552	11.131	9.740	10.336	9.044	9.272	8.441
1/17515	4.07	17.679	15.469	15.911	13.922	14.465	12.657	13.259	11.602	12.239	10.709	11.365	9.945	9.936	9.282
1/20013	3.79	16.428	14.374	14.785	12.937	13.441	11.761	12.321	10.781	11.373	9.952	10.561	9.241	9.857	8.625
1/20014	4.08	18.390	16.091	16.551	14.482	15.046	13.165	13.792	12.068	12.731	11.140	11.822	10.344	11.034	9.655
1/20015	4.37	20.284	17.749	18.256	15.974	16.596	14.522	15.213	13.312	14.043	12.288	13.040	11.410	12.171	10.649
1/20016	4.66	22.153	19.384	19.938	17.446	18.126	15.860	16.615	14.538	15.337	13.420	14.242	12.461	13.292	11.631
1/20018	5.24	25.829	22.601	23.246	20.341	21.133	18.491	19.372	16.950	17.882	15.647	16.605	14.529	15.498	13.560

These values are derived from the Ultimate Load Capacities given on pages 74-75 divided by 1.4. The tabulated values will give deflections not exceeding

span/150. The load capacities assume that the rails support a screw fixed, steel cladding system. Reference must be made to page 14 for information on bracing requirements and limitations.

For applications where any other type of cladding system is employed, please consult Tegral Technical Services Department.

background.

Zeta Sleeved Rails

All rails must be sleeved across the penultimate support (see page 5 for alternative joint arrangement diagram).

Working Load Capacities in kN

These values are derived from the Ultimate Load Capacities given on page 80 divided by 1.4. The tabulated values will give deflections not exceeding span/150. The load capacities assume that the rails support a screw fixed, steel cladding system. Reference must be made to page 14 for information on bracing requirements and limitations. For applications where any other type of cladding system is employed, please consult Tegral Technical Services Department.

The Double Span System comprises a two bay spanning purlin with simple connections at supports.

The Sleeved System comprises a single bay rail with a sleeve connection at alternate supports.

d

Sheeting line depth using standard Zeta cleats equals section depth +7mm.

General Detailing Notes

Rails may be fixed with the sheeting (wide) flange pointing up or down to suit the detail. When cladding fixed with hook bolts is employed, the sheeting (narrow) flange points up. See page 21 for minimum strut requirements.

Zeta Butted Rails

This system is employed where continuity across the supports either cannot be provided, i.e. rails are within the depth of the column,

DESIGNER'S PURLIN & RAIL GUIDE

or is not required to achieve the necessary load carrying capacity (see page 5 for alternative joint arrangement diagram).

Working Load Capacities in kN

m/na/t	m/t	4	.0	4	.5	5	5.0	5	.5	6.	0	6	.5	7	.0	7.	5	
	N N	wt kç	metr	e span	metre	span	metre	e span	metre	span								
	Secti	Self	Pressure	Suction														
1/15	5013	3.28	10.027	8.222	8.356	7.309	6.769	6.578	5.594	5.594	4.700	4.700	4.005	4.005				
1/15	5014	3.53	11.147	9.140	9.003	8.125	7.292	7.292	6.027	6.027	5.064	5.064	4.315	4.315				
1/15	5015	3.78	12.212	10.026	9.649	8.912	7.816	7.816	6.459	6.459	5.427	5.427	4.625	4.625				
1/17	7513	3.53	11.798	9.674	10.487	8.599	8.867	7.739	7.328	7.036	6.158	6.158	5.247	5.247	4.524	4.524		
1/17	7514	3.80	13.155	10.787	11.694	9.589	9.560	8.630	7.901	7.845	6.639	6.639	5.657	5.657	4.877	4.877		
1/17	7515	4.07	14.465	11.861	12.647	10.543	10.244	9.489	8.466	8.466	7.114	7.114	6.062	6.062	5.227	5.227		
1/17	7517	4.62	17.027	13.962	14.337	12.411	11.613	11.170	9.597	9.597	8.064	8.064	6.871	6.871	5.925	5.925		
1/20	0013	3.79	13.441	11.022	11.948	9.797	10.753	8.817	9.775	8.016	8.423	7.348	7.177	6.783	6.188	6.188	5.391	5.391
1/20	0014	4.08	15.046	12.338	13.374	10.967	12.037	9.870	10.808	8.973	9.082	8.225	7.738	7.593	6.672	6.672	5.812	5.812
1/20	0015	4.37	16.596	13.609	14.752	12.097	13.277	10.887	11.589	9.897	9.738	9.073	8.297	8.297	7.154	7.154	6.232	6.232
1/20	0016	4.66	18.126	14.863	16.112	13.212	14.500	11.890	12.366	10.809	10.391	9.909	8.854	8.854	7.634	7.634	6.650	6.650
1/20	0018	5.24	21.133	17.329	18.785	15.404	16.831	13.863	13.910	12.603	11.688	11.553	9.959	9.959	8.587	8.587	7.481	7.481

General notes to all Zeta cladding rail systems Working load capacities for wind pressure (wide, sheeted flange in compression at mid span) are printed on a blue background. These values are derived from the Ultimate Load Capacities given on page 80 divided by 1.4. Working load capacities for wind suction (narrow, unsheeted flange in compression at mid span) are printed on a grey background.

These values are derived from the Ultimate Load Capacities given on page 80 divided by 1.4. The tabulated values will give deflections not exceeding span/150. The load capacities assume that the rails support a screw fixed, steel cladding system. Reference must be made to page 14

for information on bracing requirements and limitations. For applications where any other type of cladding system is employed, please consult Tegral Technical Services Department.

Zeta Rail Detailing

Double Span System

Note: End bays that require a single bay purlin will need a sleeve connection at the penultimate rafter.

Sleeved and Heavy End Bay System

Note: Sleeves on every joint at penultimate column.

STANDARD	DIM	ENSION	S FOR Z	ETA SEC	TIONS
Rail depth	d	125	150	175	200
Flange widths	а	60	72	72	72
	b	50	65	65	65
Hole centres	С	62	74	87	64
	е	0	0	0	70
	f	63	76	88	66
Sleeve	g	250	315	390	405
Dimensions	h	695	825	975	1000

Firescreen Rails

All cladding and eaves rails must be allowed to expand during a fire. Slotted holes with both steel and nylon washers under the bolt heads are required. The minimum allowance for expansion should be 7mm/metre length of rail. For rails up to 7.5m span the gap between rails is rounded to 50mm. The maximum span and spacing should be 7.5m and 2.0m respectively. Rails should generally be single span, but may be sleeved to achieve continuity.

0 colum 0 column 25 25 span-50 14mm sq. strut holes 5 + + | 4 a 25 | | 25 25 100 100 g 25 125-175 SECTIONS Standard sleev Clea 25 ۲ ۲ 14mm sq. strut holes M16 nut and bolt with 2 No steel washers 7 RAIL/CLEAT CONNECTION DETAIL ______25 a 25 25 g 25 25 200 SECTIONS Typical interior bay sleeved rail for spans up to 6.1m. Typical interior bay sleeved rail for spans over 6.1m up to 7.5m.

STANDARD	DIM	ENSIONS	FOR	ZETA S	ECTIONS
Rail depth	d	125	150	175	200
Flange widths	а	60	72	72	72
	b	50	65	65	65
Hole centres	С	62	74	87	64
	е	0	0	0	70
	f	63	76	88	66
Sleeve	g	250	315	390	405
Dimensions	h	695	825	975	1000

Arrangement of Diagonal **Braces and Vertical Struts**

All rails should be braced in accordance with the recommendations shown below, to maintain alignment of the sections. Diagonal braces and vertical struts are not necessarily required to develop the load carrying capacities given on pages 16-17-18, where screw fixed steel cladding is employed. They should, however, be provided to maintain alignment during erection and cladding. Special consideration must be given to alternative cladding constructions such as clip fixed panels and glazing.

> Please consult the Technical Services Department for further information.

Maximum height 'h' for insulated Metal Sheeting (S. Wt. 13.25 kg/m2) = 12m.* Maximum height 'h' for insulated F.C. Sheeting (S. Wt. 26.75 kg/m2) = 8.5m.* For heights greater than 'h' further sets of diagonal braces are required.*

In addition, maximum number of rails per bay per set of diagonals should not exceed 8 No. for spans 9.0m (rail weight 7.94 kg/m). For other cases, please consult our Technical Services Department.

Zeta Rail Accessories

Details of diagonal brace (specify dim. 'A' and 'B' when ordering)

VS1 FOR ZETA 125, 150, 175

VS2 FOR ZETA 200

Column Stavs

The same section used for the diagonal brace channel is also available as a column stav member, where these are required by the frame designer. A special cranked version with a 20mm offset is used for Zeta cladding rails.

DESIGNER'S PURLIN & RAIL GUIDE

Zeta Cleats Purlin & Rail

Zeta Cleats Purlin & Rail

the table below.

Tegral cleats for Zeta purlins and cladding rails are available as either a weld-on type or as a bolt-on type, with the facility to offer an extended leg to suit alternative sheeting lines.

Weld-on cleats

To suit 125, 150, and 175mm purlin or rail depth

For ordering purp the Zeta cleat refe is followed by the required sheeting

eg: for a bolt-on cleat to suit a 125 purlin depth and 132mm sheeting the reference is

1/B125/132.

Now available from Patrick Lynch Roof Cladding info@patricklynchroofcladding.com www.patricklynchroofcladding.com

Design

The cleat capacities have been verified by testing at the University of Salford. For Zeta purlins and rails subject to the following constraints:

- 1. A screw fixed steel cladding system is employed.
- 2. The standard sheeting line is used.
- 3. The roof slope for purlins is not greater than 40 degrees, and cleats supporting cladding rails are horizontal, then the cleat capacity should be satisfactory.

For variations on any of the above criteria please contact Tegral Technical Services Department.

Capacity of weld-on and bolt-on type cleats

For spine in tension (direction for forces as shown for purlin) $M_{ult} M = 2.50 \text{kNm}$ For reverse condition (spine in compression as shown for rail) M_{ult} M = 1.20 kNm

To suit 200mm purlin or rail depth

ence Section	Weld-on	Bolt-on	Sheeting line Standard	Sheeting line Max	A Standard	A Max	В	C Standard	C Max
ta 1/125	1/C125/_	1/B125/_	132	287	69	224	25	45	200
nm 1/150	1/C150/_	1/B150/_	157	312	81	236	35	45	200
ne 1/175	1/C175/_	1/B175/_	182	332	94	244	45	50	200
1/200	1/C200/_	1/B200/_	207	360	71	224	25	47	197

The appropriate dimention for standard section

punching and sheeting lines, as well as limiting

dimensions for extended leg cleats are given in

Zeta 2 Purlins & Rails

Zeta 2 Purlins

Zeta 2 Purlin Section Profile

Zeta 2 Section Properties

P

nge a

77 d - 151 - "-|-"-b

Standard holing lines for all sections 18 dia. or 14 sq. web holes, and 14 dia. flange holes may be punched in any pattern on these standard lines.

	Section No	Depth d	Thickness t	Top Flange a	Btm Flange	Weight	Area	lxx	ZXX	lyy	гуу	×	٨	
_		mm	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	mm	
	2/22514	225	1.4	78	68	4.47	570	417.8	36.38	59.72	32.98	75.10	114.85	
	2/22515	225	1.5	78	68	4.79	610	448.2	39.02	64.11	32.98	75.15	114.85	
	2/22516	225	1.6	78	68	5.10	650	478.4	41.66	68.49	32.98	75.20	114.85	
	2/22518	225	1.8	78	68	5.73	731	538.7	46.90	77.23	32.98	75.30	114.85	
	2/22520	225	2.0	78	68	6.37	811	598.6	52.12	85.95	32.97	75.40	114.85	
	2/22525	225	2.5	78	68	7.94	1011	746.9	65.03	107.65	32.96	75.65	114.85	
	2/24514	245	1.4	78	68	4.69	598	510.7	40.88	59.73	32.20	75.02	124.94	
	2/24515	245	1.5	78	68	5.02	640	547.9	43.85	64.12	32.20	75.07	124.94	
	2/24516	245	1.6	78	68	5.35	682	584.9	46.81	68.50	32.20	75.12	124.94	
	2/24518	245	1.8	78	68	6.02	767	658.6	52.72	77.24	32.19	75.22	124.94	
	2/24520	245	2.0	78	68	6.68	851	732.0	58.59	85.96	32.19	75.32	124.94	
	2/24525	245	2.5	78	68	8.31	1058	913.7	73.13	107.66	32.22	75.57	124.94	
	2/26515	265	1.5	78	68	5.26	670	659.9	48.88	64.13	31.47	74.99	135.01	
	2/26516	265	1.6	78	68	5.61	714	704.5	52.18	68.50	31.47	75.04	135.01	
	2/26518	265	1.8	78	68	6.30	803	793.5	58.77	77.25	31.47	75.14	135.01	
	2/26520	265	2.0	78	68	6.99	891	882.0	65.33	85.97	31.46	75.24	135.01	
	2/26525	265	2.5	78	68	8.72	1111	1101.4	81.58	107.68	31.45	75.49	135.01	
	2/26530	265	3.0	78	68	10.44	1330	1318.0	97.62	129.25	31.44	75.74	135.02	
	2/28515	285	1.5	78	68	5.49	700	784.9	54.10	64.13	30.79	74.92	145.08	
	2/28516	285	1.6	78	68	5.86	746	838.1	57.77	68.51	30.79	74.97	145.08	
	2/28518	285	1.8	78	68	6.58	839	944.0	65.06	77.26	30.78	75.07	145.09	
	2/28520	285	2.0	78	68	7.31	931	1049.4	72.33	85.98	30.78	75.17	145.09	
	2/28525	285	2.5	78	68	9.11	1161	1310.9	90.35	107.69	30.77	75.42	145.09	
	2/28530	285	3.0	78	68	10.91	1390	1569.2	108.15	129.27	30.75	75.67	145.09	

27

This system utilises single bay purlin lengths joined with connecting sleeves at alternate supports. All purlins must be sleeved across the penultimate support (see page 5 for alternative joint

	Ē	6	5.0	6	.5	7	.0	7.	5	8	.0	8	.5	9	.0	9.	5	1(0.0
on No	vt kg,	met	re span	met	re span	metr	re span	metr	re span	met	re span	metr	re span						
Sectio	Self v	Gravity	Uplift																
2/22514	4.47	12.174	11.634	11.195	10.781	10.353	10.053	9.621	9.425	8.977	8.879								
2/22515	4.79	13.723	13.086	12.622	12.125	11.675	11.304	10.851	10.596	10.128	9.979	9.486	9.438						
2/22516	5.10	15.313	14.575	14.087	13.502	13.033	12.586	12.115	11.795	11.310	11.106	10.596	10.502	9.438	9.967				
2/22518	5.73	18.574	17.628	17.091	16.326	15.816	15.214	14.708	14.254	13.641	13.418	12.004	12.683	10.628	11.639				
2/24514	4.69	13.283	12.673	12.217	11.742	11.300	10.948	10.502	10.262	9.801	9.666	9.180	9.142	8.625	8.678				
2/24515	5.02	15.010	14.289	13.808	13.238	12.775	12.340	11.875	11.564	11.085	10.889	10.386	10.297	9.761	9.772				
2/24516	5.35	16.780	15.945	15.439	14.769	14.286	13.765	13.283	12.898	12.402	12.142	11.621	11.479	10.925	10.892				
2/24518	6.02	20.403	19.333	18.777	17.902	17.379	16.681	16.163	15.626	15.096	14.706	14.150	13.898	13.307	13.184				
2/26515	5.26	16.260	15.459	14.959	14.319	13.841	13.346	12.868	12.506	12.014	11.775	11.257	11.132	10.582	10.564	9.975	10.058	9.426	9.605
2/26516	5.61	18.216	17.287	16.762	16.010	15.511	14.919	14.424	13.978	13.469	13.158	12.624	12.437	11.869	11.800	11.190	11.232	10.577	10.724
2/26518	6.30	22.214	21.020	20.446	19.462	18.926	18.132	17.604	16.983	16.444	15.981	15.417	15.101	14.500	14.322	13.677	13.629	12.933	13.007
2/28515	5.49	16.281	16.569	16.052	15.346	14.853	14.302	13.811	13.401	12.896	12.615	12.085	11.925	11.361	11.315	10.711	10.772	10.123	10.286
2/28516	5.86	19.593	18.573	18.030	17.200	16.687	16.027	15.519	15.014	14.493	14.131	13.585	13.356	12.774	12.670	12.046	12.059	11.388	11.512
2/28518	6.58	23.976	22.662	22.069	20.981	20.431	19.545	19.006	18.304	17.756	17.222	16.649	16.272	15.661	15.431	14.774	14.682	13.972	14.010

General notes to all Zeta 2 Purlin Systems

Working load capacities for gravity loading are printed on a blue background. These values are derived from the Ultimate Load Capacities given on pages 76-77 divided by 1.6. Working load capacities for wind uplift are printed on a grey background.

These values are derived from the Ultimate Load Capacities given on pages 75-76 divided by 1.4.

Zeta 2 Sleeved Purlins

arrangement diagram). The use of single span purlin lengths ensures easier on site handling and compliance with Health & Safety Authority requirements.

Working Load Capacities in kN

The effect of purlin self weight is included in the tabulated loads above. The tabulated values will give deflections not exceeding

span/150. The load capacities assume that the rails support a screw

fixed, steel cladding system. Reference must be made to page 29 for information on bracing requirements and limitations.

For applications where any other type of sheeting system is employed, i.e. other than screw fixed sheeting, please consult Tegral Technical Services Department.

Zeta 2 Heavy End Bay Purlins

This is a fully continuous system with sleeves provided at all supports. The end bay purlins and sleeves to the penultimate support are increased in gauge to compensate for the lack of continuity across the gable, hence the name (see page 5 for alternative joint

	ction	6	.0	6.	5	7	.0	7.	5	8	.0	8.	5	9.	0	9.	5	10.	0
n No.	ay Se	metre	e span	metre	span	metre	span	metre	span	metre	span	metre	span	metre	span	metre	span	metre	span
Interic Sectio	End B	Gravity	Uplift																
2/22514	.18	16.415	15.512	15.110	14.361	13.989	13.377	13.014	12.528	12.158	11.787	11.400	11.137	10.724	10.561				
2/22515	.18	18.499	17.453	17.031	16.156	15.769	15.047	14.672	14.089	13.710	13.254	12.004	12.520	10.628	11.639				
2/22516	.20	20.638	19.444	19.002	17.996	17.597	16.759	16.375	15.690	15.158	14.758	13.339	13.938	11.809	12.934				
2/24514	.18	17.907	16.901	16.485	15.645	15.263	14.572	14.201	13.645	13.269	12.836	12.444	12.126	11.708	11.497				
2/24515	.18	20.231	19.062	18.627	17.643	17.249	16.430	16.051	15.383	15.001	14.469	14.070	13.666	13.241	12.954				
2/24516	.20	22.611	21.276	20.821	19.690	19.283	18.334	17.947	17.162	16.774	16.140	15.737	15.242	14.812	14.446				
2/26515	.18	21.894	20.610	20.160	19.074	18.670	17.761	17.376	16.627	16.240	15.638	15.234	14.768	14.338	13.998	13.533	13.311	12.806	12.696
2/26516	.20	24.522	23.052	22.583	21.332	20.917	19.861	19.469	18.590	18.199	17.482	17.075	16.507	16.073	15.643	15.173	14.874	14.361	14.184
2/26518	.25	29.894	28.041	27.535	25.944	25.508	24.150	23.748	22.600	22.204	21.247	20.838	20.057	19.620	19.003	18.527	18.063	17.541	17.220
2/28515	.18	23.480	22.086	21.622	20.439	20.026	19.031	18.638	17.814	17.421	16.753	16.344	15.820	15.384	14.993	14.522	14.257	13.743	13.596
2/28516	.20	26.363	24.764	24.280	22.914	22.490	21.333	20.935	19.966	19.571	18.774	18.364	17.725	17.288	16.797	16.322	15.969	15.450	15.226
2/28518	.25	32.249	30.226	29.706	27.963	27.522	26.028	25.625	24.355	23.961	22.896	22.489	21.612	21.177	20.474	19.999	19.459	18.936	18.549

General notes to all Zeta 2 Purlin Systems

Working load capacities for gravity loading are printed on a blue background. These values are derived from the Ultimate Load Capacities given on pages 77 divided by 1.6. Working load capacities for wind uplift are printed on a grey background.

These values are derived from the Ultimate Load Capacities given on pages 77 divided by 1.4.

Zeta 2 Butted Purlins

This system is employed where continuity across the supports either can not be provided, i.e. purlins are within the depth of the rafter, or is not required to achieve the necessary load carrying capacity. (see page 5 for alternative joint arrangement diagram).

The effect of purlin self weight is included in the tabulated

The tabulated values will give deflections not exceeding

The load capacities assume that the rails support a screw

fixed, steel cladding system. Reference must be made to

For applications where any other type of sheeting system is employed, i.e. other than screw fixed sheeting, please

page 29 for information on bracing requirements and

consult Tegral Technical Services Department.

Working Load Capacities in kN

2	m/	6	.0	6	.5	7	.0	7	.5	8.	0	8	.5	9	.0	9.	5	10	.0
on No	vt kg,	metr	e span	metre	e span	metre	e span	metre	span	metre	span	metre	e span	metre	e span	metre	span	metre	span
Sectio	Self v	Gravity	Uplift																
2/22514	4.47	8.235	8.033	7.560	7.457	6.978	6.967												
2/22515	4.79	9.288	9.032	8.528	8.382	7.671	7.829												
2/22516	5.10	10.369	10.055	9.523	9.330	8.190	8.711												
2/22518	5.73	12.586	12.153	10.786	11.272	9.222	10.009												
2/24514	4.69	8.989	8.747	8.253	8.119	7.620	7.583	7.067	7.122										
2/24515	5.02	10.164	9.858	9.334	9.147	8.620	8.541	7.998	8.019										
2/24516	5.35	11.367	10.996	10.442	10.200	9.646	9.522	8.701	8.938										
2/24518	6.02	13.830	13.323	12.710	12.355	11.343	11.529	9.799	10.684	8.529	9.474								
2/26515	5.26	11.004	10.654	10.108	9.884	9.336	9.228	8.664	8.662	8.073	8.171								
2/26516	5.61	12.333	11.908	11.332	11.045	10.469	10.309	9.718	9.675	9.057	9.124	8.061	8.641						
2/26518	6.30	15.051	14.470	13.833	13.417	12.786	12.518	11.874	11.743	10.350	11.069	9.081	10.132	8.012	9.125				
2/28515	5.49	11.806	11.412	10.846	10.586	10.019	9.882	9.299	9.275	8.666	8.748	8.104	8.286	7.601	7.878				
2/28516	5.86	13.264	12.788	12.189	11.859	11.263	11.068	10.456	10.385	9.747	9.792	9.118	9.272	8.532	8.813	7.576	8.405		
2/28518	6.58	16.243	15.592	14.931	14.455	13.802	13.484	12.820	12.648	11.956	11.920	10.879	11.281	9.613	10.717	8.536	9.762	7.611	8.902

loads above.

span/150.

limitations.

General notes to all Zeta 2 Purlin Systems

Working load capacities for gravity loading are printed on a blue background. These values are derived from the Ultimate Load Capacities given on page 78 divided by 1.6. Working load capacities for wind uplift are printed on a grey background.

These values are derived from the Ultimate Load Capacities given on page 78 divided by 1.4.

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Zeta 2 Heavy End Bay Purlins

arrangement diagram). This system is usually more advantageous for longer spans and/or heavier loadings where there are a minimum of six bays. Providing sleeves at all supports increases the capacity by over 25% compared to the equivalent sleeved system, with sleeves at alternate joints.

Working Load Capacities in kN

The effect of purlin self weight is included in the tabulated loads above.

The tabulated values will give deflections not exceeding span/150.

The load capacities assume that the rails support a screw fixed, steel cladding system. Reference must be made to page 29 for information on bracing requirements and limitations.

For applications where any other type of sheeting system is employed, i.e. other than screw fixed sheeting, please consult Tegral Technical Services Department.

Zeta 2 Purlin Detailing

DESIGNER'S PURLIN & RAIL GUIDE

Zeta 2 Purlin Detailing

Sleeved System

The sleeved System comprises a single bay purlin Note: Sleeve with a sleeve connected at alternate supports.

Note: Sleeves on every joint at penultimate rafter.

Heavy End Bay System

The Heavy End Bay System comprises a single bay purlin with a sleeve connection at every support.

Note: End Bays require the appropriate heavier guage section and sleeve at the penultimate rafter.

e de la coltra de la		
General detailing notes All purlips must be fixed with the top flance		Р
facing the apex. See page 29 for maximum		F
sag bar requirements.	7	
All fixing holes are 18 dia. for M16 grade 4.6	Sheeting line depth using	Н
bolts. Sag Bar/Bracing Strut holes are 14 sq.	standard Zeta II cleats equals section depth +7mm.	
punched in pairs.		

DIME	NSIONS	FOR ZE	TA 2 SE	CTIONS	
d	225	245	265	285	
а	78	78	78	78	
b	68	68	68	68	
С	74	74	74	74	
е	74	94	114	134	
f	77	77	77	77	
g	350	375	400	425	
h	850	900	950	1000	
	d a b c e f g h	d 225 a 78 b 68 c 74 e 74 f 77 g 350 h 850	d 225 245 a 78 78 b 68 68 c 74 74 e 74 94 f 77 77 g 350 375 h 850 900	d 225 245 265 a 78 78 78 b 68 68 68 c 74 74 74 e 74 94 114 f 77 77 77 g 350 375 400 h 850 900 950	d 225 245 265 285 a 78 78 78 78 b 68 68 68 68 c 74 74 74 74 f 77 77 77 77 g 350 375 400 425 h 850 900 950 1000

Zeta 2 Purlin Accessories

Sag Bar Requirements

Where adequate lateral restraints is afforded by steel sheeting screw fixed to the purlin flange, sag bars are required to develop the load capacities given on page 25, 26 & 27.

To maintain the correct alignment and to help prevent distortion of the purlin during sheeting, sag bars should be fitted in accordance with our minimum recommendations shown in the table below.

PI	JRLIN SPA	AN IN METRES					
Roof pitch θ	S ≤ 4.5	4.5< S ≤ 7.6	7.6 < S≼ 90				
$0^{\circ} < \theta \le 3^{\circ}$	Consult	Technical Servi	vices Dept.				
$3^{\circ} < \theta \le 30^{\circ}$	0	1*	2*				
θ > 30°	Consult	Technical Servi	ces Dept.				

For all Zeta 2 sections Heavy Duty Sag Bars should be used as required. *See section heading "Eaves Tie and Fixing Bracket" for guidance on restraint to first purlins at eaves.

For roof slope lengths in excess of 20m, it may become necessary to introduce additional support along the roof slope in the form of diagonal brace section tied back to the rafters. Details of the diagonal braces are shown on page 33. Please consult the Technical Services Department for further advice.

Special consideration must be given to purlins clad with any material other than screw fixed profiled steel sheeting. These include fibre cement sheeting, standing seam clip fix profiles and tiled roof constructions.

APEX TIE DETAILS

Details required from customer. Dimension A Dimension H Angle θ

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100 x 100 x 1.5mm thick galvanised steel verge trim angle for fixing to the top flange, available in 4m unpunched lenghts.

This system utilises single bay rail lengths joined

All rails must be sleeved across the penultimate

with connecting sleeves at alternate supports.

7.0

7.5

Zeta 2 Rails

Zeta 2 Rail Section Profile

Standard holing lines for all sections 18 dia. or 14 sq. web holes, and 14 dia. flange holes may be punched in any pattern on these standard lines.

Zeta 2 Rail Section Properties

Section No	Depth d	Thickness t	Top Flange a	Btm Flange b	Weight	Area	lxx	ZXX	lyy	ryy	×	٨	
	mm	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	mm	
2/22514	225	1.4	78	68	4.47	570	417.8	36.38	59.72	32.98	75.10	114.85	
2/22515	225	1.5	78	68	4.79	610	448.2	39.02	64.11	32.98	75.15	114.85	
2/22516	225	1.6	78	68	5.10	650	478.4	41.66	68.49	32.98	75.20	114.85	
2/22518	225	1.8	78	68	5.73	731	538.7	46.90	77.23	32.98	75.30	114.85	
2/22520	225	2.0	78	68	6.37	811	598.6	52.12	85.95	32.97	75.40	114.85	
2/22525	225	2.5	78	68	7.94	1011	746.9	65.03	107.65	32.96	75.65	114.85	
2/24514	245	1.4	78	68	4.69	598	510.7	40.88	59.73	32.20	75.02	124.94	
2/24515	245	1.5	78	68	5.02	640	547.9	43.85	64.12	32.20	75.07	124.94	
2/24516	245	1.6	78	68	5.35	682	584.9	46.81	68.50	32.20	75.12	124.94	
2/24518	245	1.8	78	68	6.02	767	658.6	52.72	77.24	32.19	75.22	124.94	
2/24520	245	2.0	78	68	6.68	851	732.0	58.59	85.96	32.19	75.32	124.94	
2/24525	245	2.5	78	68	8.31	1058	913.7	73.13	107.66	32.22	75.57	124.94	
2/26515	265	1.5	78	68	5.26	670	659.9	48.88	64.13	31.47	74.99	135.01	
2/26516	265	1.6	78	68	5.61	714	704.5	52.18	68.50	31.47	75.04	135.01	
2/26518	265	1.8	78	68	6.30	803	793.5	58.77	77.25	31.47	75.14	135.01	
2/26520	265	2.0	78	68	6.99	891	882.0	65.33	85.97	31.46	75.24	135.01	
2/26525	265	2.5	78	68	8.72	1111	1101.4	81.58	107.68	31.45	75.49	135.01	
2/26530	265	3.0	78	68	10.44	1330	1318.0	97.62	129.25	31.44	75.74	135.02	
2/28515	285	1.5	78	68	5.49	700	784.9	54.10	64.13	30.79	74.92	145.08	
2/28516	285	1.6	78	68	5.86	746	838.1	57.77	68.51	30.79	74.97	145.08	
2/28518	285	1.8	78	68	6.58	839	944.0	65.06	77.26	30.78	75.07	145.09	
2/28520	285	2.0	78	68	7.31	931	1049.4	72.33	85.98	30.78	75.17	145.09	
2/28525	285	2.5	78	68	9.11	1161	1310.9	90.35	107.69	30.77	75.42	145.09	
2/28530	285	3.0	78	68	10.91	1390	1569.2	108.15	129.27	30.75	75.67	145.09	

33

wt kg/m 9. metre span metre span metre span metre span Self Gravity Uplift Gravity Uplift Gravity Uplift Gravity Uplift Gravity 2/22514 4.47 12.174 11.634 11.195 10.781 10.353 10.053 9.621 9.425 2/22515 4.79 13.723 13.086 12.622 12.125 11.675 11.304 10.851 10.596 10 2/22516 5.10 15.313 14.575 14.087 13.502 13.033 12.586 12.115 11.795 11 2/22518 5.73 18.574 17.628 17.091 16.326 15.816 15.214 14.708 14.254 13. 2/24514 4.69 13.283 12.673 12.217 11.742 11.300 10.948 10.502 10.262 9. 2/24515 5.02 15.010 14.289 13.808 13.238 12.775 12.340 11.875 11.564 11 2/24516 5.35 16.780 15.945 15.439 14.769 14.286 13.765 13.283 12.898 12. 2/24518 6.02 20.403 19.333 18.777 17.902 17.379 16.681 16.163 15.626 15. 2/26515 5.26 16.260 15.459 14.959 14.319 13.841 13.346 12.868 12.506 12. 2/26516 5.61 18.216 17.287 16.762 16.010 15.511 14.919 14.424 13.978 13. 2/26518 6.30 22.214 21.020 20.446 19.462 18.926 18.132 17.604 16.983 16 2/28515 5.49 16.281 16.569 16.052 15.346 14.853 14.302 13.811 13.401 12. **2/28516 5.86** 19.593 18.573 18.030 17.200 16.687 16.027 15.519 15.014 14. 2/28518 6.58 23.976 22.662 22.069 20.981 20.431 19.545 19.006 18.304 17.

6.5

6.0

Zeta 2 Butted Rails - Working Load Capacities in kN

ion No.	wt kg/m	6 metr	.0 e span	6 metre	i.5 2 span	7 metre	7.0 e span	7 metre	7.5 e span	8. metre	.0 e span	8 metre	.5 2 span	9 metre	.0 e span	9. metre	5 span	10 metre	.0 e span
Sect	Self	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift
2/22514	4.47	8.235	8.033	7.560	7.457	6.978	6.967												
2/22515	4.79	9.288	9.032	8.528	8.382	7.671	7.829												
2/22516	5.10	10.369	10.055	9.523	9.330	8.190	8.711												
2/22518	5.73	12.586	12.153	10.786	11.272	9.222	10.009												
2/24514	4.69	8.989	8.747	8.253	8.119	7.620	7.583	7.067	7.122										
2/24515	5.02	10.164	9.858	9.334	9.147	8.620	8.541	7.998	8.019										
2/24516	5.35	11.367	10.996	10.442	10.200	9.646	9.522	8.701	8.938										
2/24518	6.02	13.830	13.323	12.710	12.355	11.343	11.529	9.799	10.684	8.529	9.474								
2/26515	5.26	11.004	10.654	10.108	9.884	9.336	9.228	8.664	8.662	8.073	8.171								
2/26516	5.61	12.333	11.908	11.332	11.045	10.469	10.309	9.718	9.675	9.057	9.124	8.061	8.641						
2/26518	6.30	15.051	14.470	13.833	13.417	12.786	12.518	11.874	11.743	10.350	11.069	9.081	10.132	8.012	9.125				
2/28515	5.49	11.806	11.412	10.846	10.586	10.019	9.882	9.299	9.275	8.666	8.748	8.104	8.286	7.601	7.878				
2/28516	5.86	13.264	12.788	12.189	11.859	11.263	11.068	10.456	10.385	9.747	9.792	9.118	9.272	8.532	8.813	7.576	8.405		
2/28518	6.58	16.243	15.592	14.931	14.455	13.802	13.484	12.820	12.648	11.956	11.920	10.879	11.281	9.613	10.717	8.536	9.762	7.611	8.902

General notes to all Zeta cladding rail systems

Working load capacities for wind pressure (wide, sheeted flange in compression at mid span) are printed

on a blue background. These values are derived from the

Ultimate Load Capacities given on pages 76-78 divided by 1.4.

Working load capacities for wind suction (narrow, unsheeted flange in compression at mid span) are printed on a grey

background.

Zeta 2 Sleeved Rails

support (see page 5 for alternative joint arrangement diagram). The use of single span purlin lengths ensures easier on site handling and compliance with Health & Safety Authority requirements.

Zeta 2 Sleeved Rails - Working Load Capacities in kN

8. metr	.0 e span	8.5 metre span		9. metr	.0 re span	9. meti	5 'e span	10.0 metre span		
ovity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	
977	8.879									
.128	9.979	9.486	9.438							
.310	11.106	10.596	10.502	9.438	9.967					
.641	13.418	12.004	12.683	10.628	11.639					
801	9.666	9.180	9.142	8.625	8.678					
.085	10.889	10.386	10.297	9.761	9.772					
.402	12.142	11.621	11.479	10.925	10.892					
.096	14.706	14.150	13.898	13.307	13.184					
.014	11.775	11.257	11.132	10.582	10.564	9.975	10.058	9.426	9.605	
.469	13.158	12.624	12.437	11.869	11.800	11.190	11.232	10.577	10.724	
.444	15.981	15.417	15.101	14.500	14.322	13.677	13.629	12.933	13.007	
.896	12.615	12.085	11.925	11.361	11.315	10.711	10.772	10.123	10.286	
.493	14.131	13.585	13.356	12.774	12.670	12.046	12.059	11.388	11.512	
.756	17.222	16.649	16.272	15.661	15.431	14.774	14.682	13.972	14.010	

These values are derived from the Ultimate Load Capacities given on pages 76-78 divided by 1.4. The tabulated values will give deflections not exceeding span/150. The load capacities assume that the rails support a screw fixed, steel cladding system. Reference must be made to page 33 for information on bracing requirements and limitations. For applications where any other type of cladding system is employed, please consult Tegral Technical Services Department.

The Sleeved System comprises of a single bay rail

with a sleeve connection at alternate supports.

Arrangement of Diagonal **Braces and Vertical Struts**

All rails should be braced in accordance with the recommendations shown below, to maintain alignment of the sections. Diagonal braces and vertical struts are required to develop the load capacities given on page 31. Special consideration must be given to alternative cladding

constructions such as clip fixed panels and glazing. Please consult the Technical Services Department for further information.

Maximum height 'h' for insulated Metal Sheeting (S. Wt. 13.25 kg/m^{2}) = 12m.* Maximum height 'h' for insulated F.C. Sheeting $(S. Wt. 26.75 \text{ kg/m}^2) = 8.5 \text{m}.^2$ For heights greater than 'h' further sets of diagonal braces are required.*

In addition, maximum number of rails per bay per set of diagonals should not exceed 8 No. for spans \leq 9.0m (rail weight \leq 7.94 kg/m). For other cases, please consult our Technical Services Department.

Sleeved System

Note: Sleeves on every joint at penultimate column.

Firescreen Rails

All cladding and eaves rails must be allowed to expand during a fire. Slotted holes with both steel and nylon washers under the bolt heads are required.

The minimum allowance for expansion should be 7mm/metre length of rail. For rails up to 7.5m span the gap between rails is rounded to 50mm. The maximum span and spacing should be 7.5m and 2.0m respectively. Rails should generally be single span, but may be sleeved to achieve continuity.

See page 33 for minimum strut requirements. All fixing holes are 18 dia for M16 grade 4.6 bolts. Bracing Strut holes are 14 sq, punched in pairs.

400

950

425

1000

77 min

For max cantilever

projection consult Technical Services

+ +

GABLE END DETAIL

General Detailing Notes Rails may be fixed with the sheeting

(wide) flange pointing up or down to suit the detail. When cladding fixed with hook bolts is employed, the sheeting (narrow) flange points up.

Sleeve

Dimensions

	STANDARD D	DIME	NSIONS	FOR ZE	TA 2 SE	CTIONS
	Rail depth	d	225	245	265	285
	Flange widths	а	78	78	78	78
		b	68	68	68	68
ing	Hole centres	С	74	74	74	74
equals		е	74	94	114	134
		f	77	77	77	77

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Sheeting line depth us

standard Zeta II cleats section depth +7mm.

35

350

850

Q

h

375

900

Zeta 2 Rail Accessories

Diagonal Brace, fixing bracket & vertical strut fixing

Details of diagonal brace (specify dim. 'A' and 'B' when ordering)

Column Stays

The same section used for the diagonal brace channel is also available as a column stay member, where these are required by the frame designer.

DESIGNER'S PURLIN & RAIL GUIDE

Zeta 2 Purlin & Rail Cleats

Zeta 2 Purlin & Rail Cleats

Tegral cleats for Zeta 2 purlins and cladding rails are available as either a weld-on type or as a bolt-on type, with the facility to offer an extended leg to suit alternative sheeting lines. The appropriate dimension for standard section punching and sheeting lines, as well as limiting dimensions for extended leg cleats are given in the table below.

Weld-on cleats

Design

The cleat capacities have been verified by testing at the University of Salford. For Zeta 2 purlins and rails subject to the following constraints:

- 1. A screw fixed steel cladding system is employed.
- 2. The standard sheeting line is used.
- 3. The roof slope for purlins is not greater than 30 degrees, and cleats supporting cladding rails are horizontal.
- Purlin/rail span not greater than
 9.0m then the cleat capacity should be satisfactory.

For variations on any of the above criteria please contact Tegral Technical Services Department.

Capacity of weld-on and bolt-on type cleats

For spine in tension (direction for forces as shown for purlin)

 M_{ult} M = 2.50kNm For reverse condition (spine in compression as shown for rail) $M_{ult} M = 1.20 \text{ kNm}$

For ordering purposes, the Zeta 2 cleat reference is followed by the required sheeting line, eg: for a bolt-on Zeta cleat to suit a 225mm purlin depth and a 232mm sheeting line the reference is 2/B225/232.

Section	Weld-on	Bolt-on	Sheeting line Standard	Sheeting line Max	A Standard	A Max	В	C
2/225	2/C225/_	2/B225/_	232	402	81	251	25	74
2/245	2/C245/_	2/B245/_	252	402	81	231	25	94
2/265	2/C265/_	2/B265/_	272	402	81	211	25	114
2/285	2/C285/_	2/B285/_	292	402	81	191	25	134

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37

18mm

48

Elevation

dia

100

Front

Zed Purlins

Zed Purlins

Zed Section Profile

Standard holing lines for all sections 14 dia. or 14 sq. web holes, and 14 dia. flange holes may be punched in any pattern on these standard lines. Also available 8, 12, 18 and 22mm dia. 14 x 25mm and 18 x 25mm slots. Please refer to page 4 for section properties.

Section No	Depth d	Thickness t	Top Flange a	Btm Flange b	Weight	Area	lxx	ZXX	lyy	гуу	×	~	
	mm	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	mm	
Z/12513	125	1.3	55	45	2.51	321	78.1	12.07	19.77	24.83	52.03	64.65	
Z/12514	125	1.4	55	45	2.70	345	84.1	13.01	21.33	24.86	52.08	64.65	
Z/12515	125	1.5	55	45	2.89	369	90.1	13.94	22.88	24.89	52.13	64.65	
Z/12517	125	1.7	55	45	3.27	418	102.1	15.79	25.99	24.94	52.23	64.65	
Z/12520	125	2.0	55	45	3.83	490	119.8	18.53	30.63	24.99	52.38	64.65	
Z/14013	140	1.3	58	49	2.74	350	106.0	14.68	23.62	25.99	51.91	72.21	
Z/14014	140	1.4	58	49	2.95	376	114.2	15.82	25.48	26.02	51.96	72.21	
Z/14015	140	1.5	58	49	3.15	403	122.4	16.95	27.34	26.05	52.01	72.21	
Z/14017	140	1.7	58	49	3.56	456	138.7	19.21	31.05	26.10	52.11	72.21	
Z/14020	140	2.0	58	49	4.16	535	162.9	22.57	36.60	26.15	52.26	72.20	
Z/15513	155	1.3	58	49	2.89	369	134.3	16.83	23.62	25.29	51.79	79.81	
Z/15514	155	1.4	58	49	3.11	397	144.7	18.14	25.49	25.33	51.84	79.81	
Z/15515	155	1.5	58	49	3.33	425	155.1	19.44	27.34	25.35	51.89	79.81	
Z/15517	155	1.7	58	49	3.76	481	175.8	22.03	31.06	25.40	51.99	79.81	
Z/15520	155	2.0	58	49	4.40	565	206.6	25.89	36.61	25.45	52.14	79.81	
Z/17013	170	1.3	58	49	3.04	389	166.7	19.07	23.63	24.66	51.69	87.41	
Z/17014	170	1.4	58	49	3.28	418	179.7	20.55	25.49	24.69	51.74	87.41	
Z/17015	170	1.5	58	49	3.50	448	192.6	22.03	27.35	24.71	51.79	87.41	
Z/17017	170	1.7	58	49	3.96	507	218.3	24.98	31.07	24.76	51.89	87.41	
Z/17020	170	2.0	58	49	4.63	595	256.6	29.36	36.62	24.81	52.04	87.41	
Z/18513	185	1.3	58	49	3.20	408	203.4	21.41	23.64	24.07	51.60	95.00	
Z/18514	185	1.4	58	49	3.44	439	219.2	23.07	25.50	24.09	51.65	95.00	
Z/18515	185	1.5	58	49	3.68	470	235.0	24.74	27.36	24.12	51.70	95.00	
2/18517	185	1.7	58	49	4.16	532	266.5	28.05	31.08	24.16	51.80	94.99	
2/18520	185	2.0	58	49	4.87	625	313.3	32.98	36.63	24.21	51.95	94.99	
Z/20013	200	1.3	58	49	3.35	428	244.6	23.84	23.64	23.51	51.52	102.58	
2/20014	200	1.4	58	49	3.60	460	263.6	25.70	25.51	23.54	51.57	102.57	
2/20015	200	1.5	58	49	3.86	493	282.7	27.56	27.37	23.57	51.62	102.57	
2/20017	200	1.7	58	49	4.36	558	320.5	31.25	31.08	23.60	51.72	102.57	
L/20020	200	2.0	58	49	5.10	655	377.0	36.75	36.64	23.65	51.87	102.57	
1/20025	200	2.5	58	49	6.33	816	470.1	45.83	45.87	23.70	52.12	102.57	

STA	NDA	RD DIN	IENSION	IS FOR Z	ED SECT	IONS	
Purlin depth	d	125	140	155	170	185	200
Flange widths	а	55	58	58	58	58	58
_	b	45	45	45	45	45	45
Hole centres	С	40	47	45	43	51	58
	е	42	42	61	80	80	80
	f	43	51	49	47	54	63
Sleeve	g	208	208	208	208	278	278
Dimensions	h	566	566	566	566	706	706

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39

Zed Section Properties

This is a fully continuous system with sleeves provided at all supports. The end bay purlins and sleeves to the penultimate support are increased in gauge to compensate for the lack of continuity across the gable, hence the name (see page 5 for alternative joint arrangement diagram.

ior ion No.	Bay ion	4. metre	. 5 2 span	5 metre	.0 e span	5 metr	5.5 e span	6 metre	.0 e span	6 metre	.5 e span	7. metre	.0 e span	7. metre	.5 e span
Sect	End	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift
Z/14013	.17	10.447	9.783	9.377	8.830	7.859	8.053	6.556	6.975	5.537	5.991	4.726	5.215		
Z/14014	.20	11.775	11.015	10.570	9.941	9.232	9.065	7.701	8.191	6.505	7.036	5.552	6.123		
Z/14015	.20	13.130	12.271	11.238	11.073	9.232	9.681	7.701	8.191	6.505	7.036	5.552	6.123		
Z/15513	.17	11.685	10.927	10.489	9.862	9.509	8.992	8.354	8.270	7.067	7.546	6.042	6.558	5.211	5.765
Z/15514	.20	13.185	12.317	11.837	11.115	10.732	10.133	9.808	9.318	8.305	8.631	7.101	7.705	6.125	6.773
Z/15515	.20	14.712	13.732	13.209	12.390	11.754	11.295	9.817	10.335	8.305	8.866	7.101	7.705	6.125	6.773
Z /17013	.17	12.905	12.055	11.586	10.878	10.504	9.918	9.600	9.120	8.833	8.447	7.551	7.872	6.523	7.106
Z/17014	.20	14.580	13.607	13.091	12.277	11.871	11.192	10.851	10.290	9.985	9.529	8.877	8.880	7.669	8.351
Z/17015	.20	16.286	15.186	14.625	13.700	13.262	12.487	12.124	11.479	10.369	10.629	8.877	9.513	7.669	8.351
Z/18513	.17	14.096	13.158	12.657	11.872	11.476	10.823	10.490	9.951	9.653	9.216	8.933	8.588	8.011	8.046
Z/18514	.20	15.953	14.877	14.326	13.421	12.991	12.233	11.876	11.246	10.930	10.413	10.117	9.702	9.410	9.088
Z/18515	.20	17.840	16.622	16.022	14.994	14.531	13.666	13.285	12.561	12.229	11.630	10.892	10.834	9.421	10.138
Z/20013	.17	15.252	14.227	13.695	12.836	12.419	11.700	11.352	10.757	10.448	9.961	9.670	9.281	8.993	8.694
Z/20014	.20	17.293	16.115	15.530	14.537	14.084	13.249	12.877	12.179	11.852	11.276	10.972	10.504	10.206	9.838
Z/20015	.20	19.366	18.033	17.394	16.265	15.776	14.823	14.425	13.624	13.279	12.612	12.294	11.748	11.438	11.001

General notes to all Zeta Purlin systems

Working load capacities for gravity loading are printed on a green background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.6. Working load capacities for wind uplift are printed on a grey background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.4. The effect of purlin self weight is included in the tabulated loads above.

The tabulated values will give deflections not exceeding span/180.

Zed Sleeved Purlins

This system utilises single bay purlin lengths joined with connecting sleeves at alternate supports. All purlins must be sleeved across the penultimate support (see page 5 for alternative joint

arrangement diagram). The use of single span purlin lengths ensures easier on site handling and compliance with Health & Safety Authority requirements.

Working Load Capacities in kN

ion No.	wt kg/m	4. metre	5 span	5 metre	.0 e span	5 metre	.5 e span	6 metre	.0 e span	6 metre	.5 e span	7. metre	0 span	7.5 metre	; span
Sect	Self	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift	Gravity	Uplift
Z/12513	2.51	6.657	6.348	5.359	5.605	4.395	4.666	3.659	3.955						
Z/12514	2.70	7.172	7.126	5.773	6.038	4.735	5.026	3.942	4.260						
Z/12515	2.89	7.685	7.905	6.186	6.470	5.074	5.386	4.224	4.564						
Z/12517	3.27	8.704	8.993	7.007	7.328	5.747	6.100	4.785	5.170						
Z/14013	2.74	7.734	7.303	6.935	6.598	6.006	6.024	5.010	5.332	4.231	4.581				
Z/14014	2.95	8.719	8.221	7.820	7.426	6.471	6.779	5.398	5.745	4.559	4.935				
Z/14015	3.15	9.724	9.157	8.443	8.270	6.936	7.276	5.785	6.156	4.887	5.288				
Z/14017	3.56	11.722	11.018	9.567	9.916	7.859	8.243	6.556	6.975	5.537	5.991				
Z/15513	2.89	8.653	8.155	7.760	7.367	7.028	6.724	6.380	6.191	5.397	5.742	4.614	5.011		
Z/15514	3.11	9.765	9.191	8.760	8.301	7.934	7.575	6.876	6.973	5.817	6.213	4.973	5.400		
Z/15515	3.33	10.898	10.245	9.777	9.251	8.824	8.442	7.370	7.762	6.234	6.659	5.330	5.787		
Z/15517	3.76	13.152	12.342	11.802	11.143	10.002	10.165	8.354	8.796	7.067	7.546	6.042	6.558		
Z/17013	3.04	9.558	8.995	8.574	8.124	7.766	7.414	7.090	6.825	6.516	6.328	5.764	5.905	4.979	5.427
Z/17014	3.28	10.800	10.152	9.690	9.167	8.778	8.365	8.016	7.698	7.257	7.137	6.212	6.658	5.366	5.849
Z/17015	3.50	12.066	11.327	10.827	10.227	9.810	9.330	8.959	8.586	7.780	7.958	6.660	7.141	5.754	6.269
Z/17017	3.96	14.583	13.668	13.088	12.338	11.861	11.254	10.414	10.353	8.820	9.325	7.551	8.095	6.523	7.106
Z/18513	3.20	10.441	9.817	9.367	8.865	8.486	8.089	7.749	7.445	7.122	6.903	6.583	6.440	6.113	6.041
Z/18514	3.44	11.819	11.097	10.605	10.019	9.609	9.141	8.776	8.411	8.068	7.797	7.460	7.272	6.589	6.820
Z/18515	3.68	13.219	12.397	11.863	11.192	10.750	10.209	9.820	9.393	9.030	8.705	8.168	8.118	7.065	7.606
Z/18517	4.16	16.007	14.986	14.367	13.527	13.022	12.336	11.898	11.347	10.808	10.513	9.262	9.802	8.011	8.623
Z/20013	3.35	11.299	10.613	10.138	9.583	9.185	8.743	8.388	8.046	7.711	7.459	7.128	6.958	6.621	6.526
Z/20014	3.60	12.813	12.019	11.498	10.851	10.419	9.898	9.517	9.107	8.751	8.440	8.092	7.872	7.518	7.381
Z/20015	3.86	14.352	13.448	12.880	12.139	11.673	11.071	10.664	10.185	9.808	9.438	9.070	8.800	8.429	8.250
Z/20017	4.36	17.419	16.295	15.637	14.706	14.174	13.410	12.952	12.333	11.915	11.426	11.022	10.651	9.684	9.982

General notes to all Zed Purlin systems

Working load capacities for gravity loading are printed on a green background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.6. Working load capacities for wind uplift are printed on a grey background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.4. The effect of purlin self weight is included in the tabulated loads above. Now available from The tabulated values will give deflections not exceeding span/180.

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The load capacities shown assume that the purlins support a screw fixed steel sheeting system. Reference must be made to page 41 for information on bracing requirements and limitations.

For applications where any other type of sheeting system is employed, i.e. other than screw fixed steel sheeting, please consult Tegral Technical Services Department.

Zed Heavy End Bay Purlins

This system is usually more advantageous for longer spans and/or heavier loadings where there are a minimum of six bays. Providing sleeves at all supports increases the capacity by over 25% compared to the equivalent sleeved system, with sleeves at alternate joints.

Working Load Capacities in kN

The load capacities shown assume that the purlins support a screw fixed steel sheeting system. Reference must be made to page 41 for information on bracing requirements and limitations. For applications where any other type of sheeting system

is employed, i.e. other than screw fixed steel sheeting, please consult Tegral Technical Services Department.

Zed Purlin Accessories

Zed Purlins Detailing

Sleeved System

Heavy End Bay System

The Heavy End Bay System comprises a single bay purlin with a sleeve connection at every support. Note: End Bays require the appropriate heavier guage section and sleeve at the penultimate rafter. Note: End Bays require the appropriate heavier guage section and sleeve at the penultimate rafter.

ALTERNATIVE GABLE END DETAILS

W

A

General detailing notes All purlins must be fixed th the top flange facing the apex. See page 41 for maximum sag bar requirements. I fixing holes are 14 dia. for	$\frac{1}{\sqrt{1-\frac{1}{2}}}$ Sheeting line depth using standard Zed cleats equals section depth +7mm.
M12 grade 8.8 bolts. Sag	

Bar/Bracing Strut holes are

14 sq. punched in pairs.

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STA	NDA	RD DIN	ENSION	IS FOR Z	ED SECT	IONS	
Purlin depth	d	125	140	155	170	185	200
Flange widths	а	55	58	58	58	58	58
	b	49	49	49	49	49	49
Hole centres	С	40	47	45	43	51	58
	е	42	42	61	80	80	80
	f	43	51	49	47	54	63
Sleeve	g	208	208	208	208	278	278
Dimensions	h	566	566	566	566	706	706

Sag Bar Requirements

Where adequate lateral restraints is afforded by steel sheeting screw fixed to the purlin flange, sag bars are required to develop the load capacities given on page 38 & 39.

To maintain the correct alignment and to help prevent distortion of the purlin during sheeting, sag bars should be fitted in accordance with our minimum recommendations shown in the table below.

PL	IRLIN SPAN IN METR	RES								
Roof pitch θ	Roof pitch θ S \leq 4.5 4.5 \leq 5 \leq 7.6 7.6 \leq S \leq 90									
$0^{\circ} < \theta \le 3^{\circ}$	$^{\circ} < \theta \le 3^{\circ}$ Consult Technical Services Dept.									
$3^\circ < \theta \le 30^\circ$	≤ 30° 0 1 [*] 2 [*]									
θ ≤ 30°	Consult Technical Services Dept.									

For purlin spacings in excess of 2m, Heavy Duty Sag Bars should be used as required *See section heading "Eaves Tie and Fixing Bracket" for guidance

on restraint to first purlins at eaves.

ARRANGEMENT OF SAG BARS

For roof slope lengths in excess of 20m, it may become necessary to introduce additional support along the roof slope in the form of diagonal brace section tied back to the rafters. Details of the diagonal braces are shown on page 47. Please consult the Technical Services Department for further advice.

Special consideration must be given to purlins clad with any material other than screw fixed profiled steel sheeting. These include fibre cement sheeting, standing seam clip fix profiles and tiled roof constructions.

Details required from customer Dimension A Dimension H Angle θ

Eaves tie and fixing bracket To ensure the first purlin at eaves is kept straight, particularly during the early stages of sheeting, it is necessary to give consideration to additional restraint to this purlin, especially on spans of 5m or more. This may be done by providing an eaves tie of the type shown, or in agreement with the sheeting contractor, a temporary prop may be used. 12 dia bolt through Eaves Tie 22 18mm diameter holes 22 35 20 overall length as required 18mm diameter holes 65 42 FB Fixing Bracket |<u>25</u>| 48 35 bottom purlin

Rafter Stays

The same section used to form the eaves tie is also available as a rafter stay member, where these are specified by the frame designer.

Cleader Members

50 x 50 x 1.5mm thick galvanised steel Cleader Angle, avaliable in 4m unpunched length.

100 x 100 x 1.5mm thick galvanised steel verge trim angle for fixing to the top flange, available in 4m unpunched lenghts.

Zed Rails

Zed Section Profile

Standard holing lines for all sections

14 dia. or 14 sq. web holes, and 14 dia. flange holes may be punched in any pattern on these standard lines.

Also available 8, 12, 18 and 22mm dia.

14 x 25mm and 18 x 25mm slots.

Please refer to page 43 for section properties.

f	80	С
	d	
17	70-200 RA	NGE

STA	STANDARD DIMENSIONS FOR ZED SECTIONS									
Purlin depth	d	125	140	155	170	185	200			
Flange widths	а	55	58	58	58	58	58			
	b	49	49	49	49	49	49			
Hole centres	С	40	47	45	43	51	58			
	е	42	42	61	80	80	80			
	f	43	51	49	47	54	63			
Sleeve	g	208	208	208	208	278	278			
Dimensions	h	566	566	566	566	706	706			

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	-	
700	SO	ct i
LCU	26	ιu

Section No	Depth d	Thickness t	Top Flange a	Btm Flange b	Weight	Area	lxx	ZXX	lyy	гуу	×	٨	
	mm	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	mm	
Z/12513	125	1.3	55	45	2.51	321	78.1	12.07	19.77	24.83	52.03	64.65	
Z/12514	125	1.4	55	45	2.70	345	84.1	13.01	21.33	24.86	52.08	64.65	
Z/12515	125	1.5	55	45	2.89	369	90.1	13.94	22.88	24.89	52.13	64.65	
Z/12517	125	1.7	55	45	3.27	418	102.1	15.79	25.99	24.94	52.23	64.65	
Z/12520	125	2.0	55	45	3.83	490	119.8	18.53	30.63	24.99	52.38	64.65	
Z/14013	140	1.3	58	49	2.74	350	106.0	14.68	23.62	25.99	51.91	72.21	
Z/14014	140	1.4	58	49	2.95	376	114.2	15.82	25.48	26.02	51.96	72.21	
Z/14015	140	1.5	58	49	3.15	403	122.4	16.95	27.34	26.05	52.01	72.21	
Z/14017	140	1.7	58	49	3.56	456	138.7	19.21	31.05	26.10	52.11	72.21	
Z/14020	140	2.0	58	49	4.16	535	162.9	22.57	36.60	26.15	52.26	72.20	
Z/15513	155	1.3	58	49	2.89	369	134.3	16.83	23.62	25.29	51.79	79.81	
Z/15514	155	1.4	58	49	3.11	397	144.7	18.14	25.49	25.33	51.84	79.81	
Z/15515	155	1.5	58	49	3.33	425	155.1	19.44	27.34	25.35	51.89	79.81	
Z/15517	155	1.7	58	49	3.76	481	175.8	22.03	31.06	25.40	51.99	79.81	
Z/15520	155	2.0	58	49	4.40	565	206.6	25.89	36.61	25.45	52.14	79.81	
Z/17013	170	1.3	58	49	3.04	389	166.7	19.07	23.63	24.66	51.69	87.41	
Z/17014	170	1.4	58	49	3.28	418	179.7	20.55	25.49	24.69	51.74	87.41	
Z/17015	170	1.5	58	49	3.50	448	192.6	22.03	27.35	24.71	51.79	87.41	
Z/17017	170	1.7	58	49	3.96	507	218.3	24.98	31.07	24.76	51.89	87.41	
Z/17020	170	2.0	58	49	4.63	595	256.6	29.36	36.62	24.81	52.04	87.41	
Z/18513	185	1.3	58	49	3.20	408	203.4	21.41	23.64	24.07	51.60	95.00	
Z/18514	185	1.4	58	49	3.44	439	219.2	23.07	25.50	24.09	51.65	95.00	
Z/18515	185	1.5	58	49	3.68	470	235.0	24.74	27.36	24.12	51.70	95.00	
Z/18517	185	1.7	58	49	4.16	532	266.5	28.05	31.08	24.16	51.80	94.99	
Z/18520	185	2.0	58	49	4.87	625	313.3	32.98	36.63	24.21	51.95	94.99	
Z/20013	200	1.3	58	49	3.35	428	244.6	23.84	23.64	23.51	51.52	102.58	
Z/20014	200	1.4	58	49	3.60	460	263.6	25.70	25.51	23.54	51.57	102.57	
Z/20015	200	1.5	58	49	3.86	493	282.7	27.56	27.37	23.57	51.62	102.57	
Z/20017	200	1.7	58	49	4.36	558	320.5	31.25	31.08	23.60	51.72	102.57	
Z/20020	200	2.0	58	49	5.10	655	377.0	36.75	36.64	23.65	51.87	102.57	
Z/20025	200	2.5	58	49	6.33	816	470.1	45.83	45.87	23.70	52.12	102.57	

tion Properties

This system is employed where continuity across the supports either cannot be provided, i.e. rails are within the depth of the column,

	m/	4.	.5	5	.0	5	5.5	6	.0	6.	5	7	.0	7	.5
N No	vt kg,	metre	e span	metre	e span	metre	metre span		e span	metre	span	metre	e span	metre	e span
Sectio	Self v	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction
Z/14013	2.74	5.495	4.923	4.451	4.431	3.679	3.679	3.091	3.091						
Z/14014	2.95	5.921	5.546	4.796	4.796	3.963	3.963	3.330	3.330						
Z/14015	3.15	6.345	6.182	5.140	5.140	4.248	4.248	3.569	3.569						
Z/15513	2.89	6.879	5.503	5.639	4.953	4.660	4.503	3.916	3.916	3.337	3.337				
Z/15514	3.11	7.502	6.206	6.077	5.586	5.022	5.022	4.220	4.220	3.596	3.596				
Z/15515	3.33	8.041	6.922	6.513	6.230	5.383	5.383	4.523	4.523	3.854	3.854				
Z/15517	3.76	9.113	8.347	7.382	7.382	6.101	6.101	5.126	5.126	4.368	4.368				
Z/17013	3.04	7.593	6.074	6.834	5.467	5.784	4.970	4.860	4.556	4.141	4.141	3.571	3.571		
Z/17014	3.28	8.575	6.860	7.542	6.174	6.233	5.613	5.238	5.145	4.463	4.463	3.848	3.848		
Z/17015	3.50	9.574	7.659	8.085	6.893	6.682	6.267	5.615	5.615	4.784	4.784	4.125	4.125		
Z/17017	3.96	11.315	9.250	9.166	8.325	7.575	7.568	6.365	6.365	5.423	5.423	4.676	4.676		
Z/18513	3.20	8.291	6.633	7.462	5.970	6.784	5.427	5.929	4.975	5.052	4.592	4.356	4.264	3.795	3.795
Z/18514	3.44	9.379	7.503	8.441	6.753	7.606	6.139	6.391	5.627	5.446	5.194	4.696	4.696	4.090	4.090
Z/18515	3.68	10.484	8.387	9.435	7.548	8.154	6.862	6.852	6.290	5.838	5.806	5.034	5.034	4.385	4.385
Z/18517	4.16	12.684	10.148	11.187	9.133	9.246	8.303	7.769	7.611	6.620	6.620	5.708	5.708	4.972	4.972
Z/20013	3.35			8.071	6.457	7.337	5.870	6.726	5.381	6.076	4.967	5.239	4.612	4.563	4.305
Z/20014	3.60			9.147	7.317	8.315	6.652	7.622	6.098	6.549	5.629	5.647	5.227	4.919	4.878
Z/20015	3.86			10.239	8.192	9.309	7.447	8.241	6.826	7.022	6.301	6.054	5.851	5.274	5.274
Z/20017	4.36			12.418	9.934	11.122	9.031	9.345	8.279	7.963	7.642	6.866	6.866	5.981	5.981

General notes to all Zed Rail systems

Working load capacities for wind pressure (wide sheeted flange in compression at mid span) are printed on a green background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.6. Working load capacities for wind suction (narrow, unsheeted flange in compression at mid span) are printed on a grey background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.4. The tabulated values will give deflections not exceeding span/150.

Zed Rails

This system utilises single bay rail lengths joined with connecting sleeves at alternate supports.

All rails must be sleeved across the penultimate support (see page 5 for alternative joint arrangement diagram).

The load capacities shown assume that the rail support a

screw fixed steel cladding system. Reference must be

made to page 47 for information on bracing

sheeting system is employed, please consult

For applications where any other type of

Tegral Technical Services Department.

requirements and limitations.

Working Load Capacities in kN

ö	m/l	4.	5	5.	.0	5	.5	6	.0	6.	.5	7.	0	7.5	5
N N	wt kg	metre	span	metre	e span	metre	e span	metre	e span	metre	e span	metre	span	metre	span
Secti	Self	Pressure	Suction												
Z/12513	2.51	7.796	6.237	6.579	5.613	5.437	5.103	4.569	4.569						
Z/12514	2.70	8.749	7.006	7.087	6.306	5.857	5.733	4.921	4.921						
Z/12515	2.89	9.375	7.777	7.593	6.999	6.276	6.276	5.273	5.273						
Z/12517	3.27	10.618	9.538	8.601	8.584	7.108	7.108	5.973	5.973						
Z/14013	2.74	8.978	7.182	8.080	6.464	7.345	5.876	6.205	5.387	5.287	4.972				
Z/14014	2.95	10.113	8.091	9.102	7.282	7.956	6.620	6.685	6.068	5.696	5.601				
Z/14015	3.15	11.272	9.018	10.145	8.116	8.527	7.378	7.165	6.763	6.105	6.105				
Z/15513	2.89	10.034	8.028	9.031	7.225	8.210	6.568	7.526	6.021	6.698	5.558	5.775	5.161		
Z/15514	3.11	11.317	9.054	10.185	8.148	9.259	7.408	8.471	6.790	7.218	6.268	6.224	5.820		
Z/15515	3.33	12.622	10.098	11.360	9.088	10.327	8.262	9.079	7.573	7.736	6.991	6.670	6.491		
Z/15517	3.76	15.220	12.176	13.698	10.959	12.246	9.962	10.290	9.132	8.768	8.430	7.560	7.560		
Z/17013	3.04	11.076	8.861	9.969	7.975	9.062	7.250	8.307	6.646	7.668	6.135	7.121	5.696	6.244	5.317
Z/17014	3.28	12.509	10.007	11.258	9.006	10.234	8.188	9.382	7.505	8.660	6.928	7.725	6.433	6.729	6.004
Z/17015	3.50	13.966	11.173	12.569	10.056	11.427	9.141	10.475	8.380	9.604	7.735	8.281	7.183	7.213	6.704
Z/17017	3.96	16.866	13.493	15.180	12.144	13.800	11.040	12.650	10.120	10.887	9.341	9.387	8.674	8.177	8.096
Z/18513	3.20	12.095	9.676	10.885	8.708	9.896	7.916	9.071	7.257	8.373	6.699	7.775	6.220	7.257	5.805
Z/18514	3.44	13.681	10.945	12.313	9.851	11.194	8.955	10.261	8.209	9.472	7.577	8.795	7.036	8.209	6.567
Z/18515	3.68	15.293	12.235	13.764	11.011	12.513	10.010	11.470	9.176	10.588	8.470	9.831	7.865	8.802	7.341
Z/18517	4.16	18.503	14.803	16.653	13.322	15.139	12.111	13.878	11.102	12.810	10.248	11.457	9.516	9.981	8.882
Z/20013	3.35	13.082	10.465	11.774	9.419	10.703	8.563	9.811	7.849	9.057	7.245	8.410	6.728	7.849	6.279
Z/20014	3.60	14.825	11.860	13.343	10.674	12.130	9.704	11.119	8.895	10.264	8.211	9.530	7.624	8.895	7.116
Z/20015	3.86	16.596	13.277	14.937	11.949	13.579	10.863	12.447	9.958	11.490	9.192	10.669	8.535	9.958	7.966
Z/20017	4.36	20.128	16.102	18.115	14.492	16.468	13.174	15.096	12.077	13.935	11.148	12.939	10.351	12.006	9.661

General notes to all Zed Rail systems

Working load capacities for wind pressure (wide sheeted flange in compression at mid span) are printed on a green background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.6. Working load capacities for wind suction (narrow, unsheeted flange in compression at mid span) are printed on a grey background. These values are derived from the Ultimate Load Capacities given on pages 79-80 divided by 1.4. The tabulated values will give deflections not exceeding span/150.

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Zed Rails

or is not required to achieve the necessary load carrying capacity (see page 5 for alternative joint arrangement diagram).

Working Load Capacities in kN

The load capacities shown assume that the rail support a screw fixed steel cladding system. Reference must be made to page 47 for information on bracing requirements and limitations. For applications where any other type of sheeting system is employed, please consult Tegral Technical Services Department.

Zed Rail Detailing

Sleeved System

The sleeved System comprises a single bay rail with a sleeve connection at alternate supports. Note: Sleeves on every joint at penultimate column

Firescreen Rails

All cladding and eaves rails must be allowed to expand during a fire. Slotted holes with both steel and nylon washers under the bolt heads are required. The minimum allowance for expansion should be 7mm/metre length of rail. For rails up to 7.5m span the gap between rails is rounded to 50mm.

The maximum span and spacing should be 7.5m and 2.0m respectively. Rails should generally be single span, but may be sleeved to achieve continuity.

Bracing Strut holes are 14 sq. punched in pairs.

RAIL/CLEAT CONNECTION DETAIL

⊈ colum

See page 47 for minimum strut requirements. All fixing holes are 14 dia for M12 grade 8.8 bolts.

General Detailing Notes

Rails may be fixed with the sheeting (wide) flange pointing up or down to suit the detail. When cladding fixed with hook bolts is employed, the sheeting (narrow) flange points up.

Sheeting line depth using standard Zed cleats equal section depth +7mm

STANDARD DIMENSIONS FOR ZED SECTIONS 125 Rail depth d 140 155 170 185 200 Flange widths 55 58 58 58 58 58 а 49 49 49 49 49 49 51 58 47 45 43 Hole centres С 40 42 42 61 80 80 80 ρ 43 51 49 47 54 63 Sleeve 208 208 208 208 278 278 g Dimensions 566 566 566 566 706 706 h

Arrangement of Diagonal

alignment of the sections. Diagonal braces and vertical struts are required to develop the load capacities given on pages 44 & 45. Special consideration must be given to alternative cladding constructions such as clip fixed panels and glazing. Please consult the Technical Services Department for further information.

Maximum height 'h' for insulated Metal Sheeting $(S. Wt. 13.25 \text{ kg/m}^2) = 12\text{m.}^{\circ}$ Maximum height 'h' for insulated F.C. Sheeting $(5. Wt. 26.75 \text{ kg/m}^2) = 8.5 \text{m}.^3$ For heights greater than 'h' further sets of diagonal braces are required. * In addition, maximum number of rails per bay per set of diagonals should not exceed 8 No. for spans \leq 9.0m. For other cases, please consult our Technical Services Department.

GABLE END DETAIL

+ ! +

68 min.

For max cantilever

projection consult Technical Services

Zed Rail Accessories

Diagonal Brace, fixing bracket & vertical strut fixing.

The top of the diagonal brace must be fixed back to a "rigid" point, generally the stanchion flange. Where the diagional brace is tied back to the stanchion via the cladding rail cleat, i.e. using an FB bracket as illustrated, the structural adequacy of the cleat must be confirmed.

VS1 FOR ALL SECTIONS

Details of diagonal brace (specify dim. 'A' and 'B' when ordering)

Column Stays

The same section used for the diagonal brace channel is also available as a column stay member, where these are required by the frame designer.

DESIGNER'S PURLIN & RAIL GUIDE

Zed Purlins & Rails Cleats

Zed Purlins & Rails Cleats

Tegral cleats for Zed purlins and cladding rails are available as either a weld-on type or as a bolt-on type, with the facility to offer an extended leg to suit alternative sheeting lines. The appropriate dimension for standard section punching and sheeting lines, as well as limiting dimensions for extended leg cleats are given in the table below.

Weld-on cleats

14mm

R

c (

A

Elevation

dia.

152

Plan

100

Front

Design

The cleat capacities have been verified by testing at the University of Salford. For Zed purlins and rails subject to the following constraints:

- 1. A screw fixed steel cladding system is employed.
- 2. The standard sheeting line is used.

3. The roof slope for purlins is not greater than 30 degrees, and cleats supporting cladding rails are horizontal.

4. Purlin/rail span not greater than 9.0m then the cleat capacity should be satisfactory.

For variations on any of the above criteria please contact Tegral Technical Services Department.

Capacity of weld-on and bolt-on type cleats

For spine in tension

(direction for forces as shown for purlin) M_{ult} M = 2.50kNm For reverse condition (spine in compression as shown for rail) $M_{ult} M = 1.20 \text{ kNm}$

Bolt-on cleats

18mm

For ordering purposes the Zed cleat reference is followed by the required sheeting line eq: for a bolt-on Zed cleat to suit a 140mn purlin depth and a 147mm sheeting line the reference is Z/B140/147

e e	Section	Weld-on	Bolt-on	Sheeting line Standard	Sheeting line Max	A Standard	A Max	В	C
	Z/125	Z/C125/_	Z/B125/_	132	368	47	283	25	42
1	Z/140	Z/C140/_	Z/B140/_	147	376	54	283	25	42
	Z/155	Z/C155/_	Z/B155/_	162	374	52	264	25	61
	Z/170	Z/C170/_	Z/B170/_	177	372	50	245	25	80
	Z/185	Z/C185/_	Z/B185/_	192	379	58	245	25	80
	Z/200	Z/C200/_	Z/B200/_	207	387	65	245	25	80

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Eaves Beam

Load Capacities

The following design charts show allowable load capacities based on single spanning members with an eaves tie at mid span for spans up to 6.0m, and eaves ties at third points for spans of 6.0m and over. Load capacities vary for other eaves tie configurations.

For vertical loading in the absence of horizontal loading, a load factor of 1.6 has been used. For combined horizontal and vertical loading, a load factor of 1.2 has been used. For horizontal loading in the absence of vertical loading, a load factor of 1.4 has been used. Deflections are limited to span/200 for vertical loads. The following design information assumes adequate restraint in both axis. For a given span condition, load cases below the relevant span line pass, load cases above the relevant span line fail.

Design Equations

The following design equations allow a more detailed analysis of Eaves Beam performance using limit state design methods.

Eaves Beam

range of accessory components and brackets to complement fully our existing comprehensive range of Zeta, Zeta 2 and Zed purlin and cladding rail systems.

240 Eaves Beam Section Profile

Tegral's range of Eaves Beam sections are designed to combine the function of purlin, cladding rail and gutter support at the eaves position. The sections are 160mm and 240mm deep and are available in three gauges, 2.0mm, 2.5mm and 3.2mm thick (by special order). They are supported by a complete

160 Eaves Beam Section Profile

Section Properties

Section	mm Depth	t mm	kg/m	VICE MM ²	X cm⁴	XX cm ³	XX mm	K cm⁴	MZ cm ³	Å mm	ХЭ kNm	Лум kNm	
EB16020	160	2.0	6.04	752	305.18	38.16	63.71	72.04	12.55	30.95	12.88	4.89	
EB16025	160	2.5	7.50	944	378.32	47.30	63.32	88.21	15.40	30.58	17.81	6.01	
*EB16032	160	3.2	9.50	1198	476.61	59.59	63.08	110.19	19.21	30.33	23.29	7.49	
EB24020	240	2.0	7.29	908	787.10	65.60	93.11	73.98	12.43	28.55	21.21	4.85	
EB24025	240	2.5	9.07	1140	979.91	81.68	92.73	90.01	15.23	28.10	29.60	5.94	
*EB24032	240	3.2	11.51	1450	1238.37	103.22	92.42	112.75	19.02	27.89	40.03	7.42	

*Special order

Note Section properties are calculated based on a 'flat top' Eaves Beam (θ =0°), and are therefore conservative for θ values greater than zero. *Note EB 16032 and EB 24032 Now available from by Special order Patrick Lynch Roof Cladding info@patricklynchroofcladding.com 53 www.patricklynchroofcladding.com

Holing lines for all sections

18mm dia holes may be punched in any pattern along the lines indicated above.

Stress

For Eaves Beams with no eaves ties	$\frac{\text{VL}}{8\text{Mcx}} + \frac{\text{HL}}{8\text{Mcy}} = 1.0$
For Eaves Beams with eaves ties at mid-span	$\frac{VL}{8Mcx} + \frac{HL1.0}{32Mcy} = 1.0$
For Eaves Beams with eaves ties at third points	$\frac{VL}{8Mcx} + \frac{HL1.0}{90Mcy} = 1.0$
Vertical Deflection	
Deflection limited capacity (kN)	157.44 lxx d L ²
Variables	
V = Factored vertical load (kN)	

Н = Factored horizontal load (kN)L = Span (m)

Mcx = Major axis ultimate moment capacity (kNm)

Mcy = Minor axis ultimate moment capacity (kNm)

Ixx = Major axis second moment of area (cm⁴)

d = Deflection ratio (i.e. for an L/200 deflection limit, d = 200) Youngs modulus, in deflection equation above, is taken as 205 kN/mm²

54

Fixing Cleats (by special order) Type S16

Stand off fixing arrangement

Fixing Cleats (by special order) Type S24

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Stand off detail

The detail shown is for the stand off arrangement using one eaves tie at mid span. Note: additional eaves ties may be required. The top flange slope must be specified on all manufacturing details.

All hole sizes are 18 diameter to suit M16 bolts.

BRACKET TYPE: S16 & S24										
160 Ref	240 Ref	Dim A	Standard							
			Sheeting							
			Roof							
\$16/125	\$24/125	110	132							
S16/140	S24/140	125	147							
S16/150	\$24/150	135	157							
S16/155	S24/155	140	162							
S16/170	S24/170	155	177							
S16/175	S24/175	160	182							
S16/185	S24/185	170	192							
S16/200	\$24/200	185	207							
\$16/225	\$24/225	210	232							
S16/245	\$24/245	230	252							
S16/265	\$24/265	250	272							
\$16/285	\$24/285	270	292							
	BRACKE 160 Ref 516/125 516/140 516/155 516/170 516/175 516/175 516/200 516/225 516/245 516/265 516/285	BRACKET TYPE: \$16 160 Ref 240 Ref 516/125 \$24/125 \$16/140 \$24/140 \$16/150 \$24/150 \$16/150 \$24/150 \$16/155 \$24/150 \$16/155 \$24/170 \$16/175 \$24/170 \$16/175 \$24/170 \$16/175 \$24/175 \$16/185 \$24/200 \$16/255 \$24/200 \$16/255 \$24/200 \$16/255 \$24/225 \$16/255 \$24/245 \$16/255 \$24/255 \$16/255 \$24/265 \$16/255 \$24/265 \$16/265 \$24/265 \$16/265 \$24/285	BRACKET TYPE: S16 & S24 160 Ref 240 Ref Dim A 160 Ref 240 Ref Dim A 516/125 S24/125 110 S16/125 S24/140 125 S16/150 S24/150 135 S16/155 S24/155 140 S16/170 S24/170 155 S16/175 S24/175 160 S16/175 S24/175 160 S16/175 S24/175 160 S16/175 S24/185 170 S16/200 S24/200 185 S16/225 S24/225 210 S16/225 S24/225 230 S16/225 S24/245 230 S16/265 S24/265 250 S16/265 S24/265 250 S16/285 S24/285 270							

Eaves tie and fixing bracket Note: The eaves tie assembly

illustrated is not deemed to act as a lateral restraint to an unrestrained eaves beam section (see page 69).

See pages 14, 29 or 41 as appropriate for eaves tie and bracket details.

55

Fixing Cleats (by special order) (FR16 opposite hand)

Fixing Cleats Type FL24 (by special order) (FR24 opposite hand)

Flush Face Detailing

Flush face detail

The detail shown is for the flush face arrangement using one eaves tie at mid span. Note: additional eaves ties may be required. The top flange slope must be specified on all manufacturing details. All hole sizes are 18 diameter to suit M16 bolts.

Eaves tie and fixing bracket

Note: The eaves tie assembly illustrated is not deemed to act as a lateral restraint to an unrestrained eaves beam section (see page 69).

See pages 14, 29 or 41 as appropriate for eaves tie and bracket details.

Tegral 'C' Sections, have been introduced to offer the customer a wider choice and greater versatility.

The sections are available from 90mm to 160mm in depth and are supplied in two gauges, 2.5mm and 3.2mm thick.

The new range can be employed in applications such as door sections, floor members and strutting sections to suit individual customer requirements.

ection	epth		/eight	rea	×	X	ŏ	2	\$	2		eff	ŭ	lcy	
Ś	0	÷	5	<	<u>م</u>	ы,	2	£,	Ň,	5	×	< ۲	2	2	
	mm	mm	kg/m	mm'	CM⁴	cm,	mm	CM∜	cm,	mm	mm	mm²	kNm	kNm	
C9025	90	2.5	5.72	729	101.46	22.45	37.68	78.44	15.11	33.13	39.32	697	8.40	5.89	
*C9032	90	3.2	7.24	922	126.84	28.19	37.37	97.70	18.81	32.80	39.65	905	10.99	7.33	
C10525	105	2.5	6.02	766	143.80	27.37	43.74	83.20	15.49	33.27	37.52	709	10.23	6.04	
*C10532	105	3.2	7.62	970	180.23	34.33	43.43	103.69	19.30	32.94	37.84	942	13.39	7.52	
C14025	140	2.5	6.71	854	276.17	39.42	57.43	92.67	16.17	33.27	33.93	713	14.80	6.30	
*C14032	140	3.2	8.50	1083	347.54	49.65	57.11	115.59	20.17	32.93	34.25	983	19.38	7.86	
C16025	160	2.5	7.10	904	374.49	46.63	65.01	97.25	16.47	33.13	32.19	713	17.34	6.42	
*C16032	160	3.2	9.00	1147	472.02	59.00	64.67	121.35	20.55	32.79	32.52	987	23.04	8.01	
kc															

*Special order

'C' & 'CW' Sections

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'C' Sections

Ø	Specifica Section	tion Grade S390GD+Z275, refer to page 80 for full technical specification
Ø	Bolts	M16 x 45 Grade 4.6 minimum to BS EN 24018, nuts to BS EN 24034 (should be protected against corrosion)
Ø	Design	BS5950:Part 5:1998

18mm dia web and flange holes may be punched in any pattern along these lines. Note: alternative lines and sizes are available.

Section Properties

'CW' Sections

'CW' Section Properties

9		st			Dimensions f and c are shown on page 57.								
Section N	Depth d	Thicknes	Weight	Area	lxx	ZXX	lyy	гуу	×	MCX			
	mm	mm	kg/m	mm ²	cm ⁴	cm ³	cm ⁴	mm	mm	kNm			
CW12715	127	1.50	3.17	404	105.5	16.62	22.70	23.88	21.25	5.68			
CW12716	127	1.60	3.38	430	112.4	17.70	24.13	23.83	21.25	6.26			
CW12718	127	1.80	3.80	484	125.9	19.84	26.92	23.74	21.24	7.37			
CW12720	127	2.00	4.22	538	139.3	21.94	29.65	23.64	21.24	8.40			
CW14015	140	1.50	3.32	423	131.9	18.84	23.46	23.72	20.32	6.34			
CW14016	140	1.60	3.54	451	140.5	20.07	24.93	23.67	20.32	6.99			
CW14018	140	1.80	3.98	508	157.5	22.51	27.83	23.58	20.32	8.24			
CW15515	155	1.50	3.50	446	166.7	21.51	24.26	23.52	19.35	7.09			
CW15518	155	1.80	4.20	535	199.2	25.71	28.77	23.37	19.35	9.24			
CW17015	170	1.50	3.67	468	206.4	24.29	24.95	23.29	18.47	7.84			
CW17018	170	1.80	4.41	562	246.8	29.04	29.62	23.14	18.47	10.24			
CW18515	185	1.50	3.85	491	251.4	27.18	25.63	23.06	17.67	8.58			
CW18518	185	1.80	4.62	589	300.6	32.51	30.40	22.91	17.68	11.24			
CW20015	200	1.50	4.03	513	301.7	30.18	26.23	22.81	16.94	9.31			
CW20016	200	1.60	4.30	547	321.6	32.17	27.88	22.76	16.94	10.30			
CW20018	200	1.80	4.83	616	361.0	36.11	31.11	22.66	16.95	12.24			
CW20020	200	2.00	5.37	684	399.9	40.00	34.26	22.56	16.95	14.07			
CW22015	220	1.50	4.26	543	377.7	34.34	26.95	22.49	16.06	10.35			
CW22016	220	1.60	4.55	579	402.6	36.61	28.64	22.44	16.06	11.36			
CW22018	220	1.80	5.12	652	452.1	41.11	31.96	22.34	16.07	13.55			
Now availer 2020	220	2.00	5.68	724	501.0	45.56	35.21	22.24	16.08	15.63			
Patrick Lynchron	240	2.00	6.00	764	616.3	51.37	36.05	21.91	15.29	17.17			

be punched in any pattern on these lines.

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This system utilises single bay rail lengths joined with connecting sleeves at alternate supports.

'CW' Sleeved Rail - Working Load Capacities in kN

on No.	vt kg/m	4. metre	. 5 e span	5 metre	.0 e span	5 metre	.5 e span	6. metre	.0 e span	6 metr	.5 e span	7. metre	.0 e span	7.5 metre span	
Secti	Selfv	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction
CW12715	3.17	10.679	7.661	8.650	6.895	7.149	6.268	6.007	5.746						
CW12716	3.38	11.375	8.257	9.214	7.431	7.614	6.756	6.398	6.193						
CW12718	3.80	12.747	9.392	10.325	8.453	8.533	7.685	7.170	7.044						
CW12720	4.22	14.033	10.446	11.420	9.402	9.438	8.547	7.931	7.835						
CW14015	3.32	11.522	8.490	10.370	7.641	8.936	6.947	7.509	6.368	6.398	5.878				
CW14016	3.54	12.557	9.158	11.301	8.242	9.519	7.493	7.999	6.869	6.816	6.340				
CW14018	3.98	14.075	10.436	12.668	9.392	10.672	8.538	8.968	7.827	7.641	7.225				
CW15515	3.50	12.524	9.843	11.271	8.859	10.247	8.054	9.393	7.382	8.087	6.815	6.973	6.328		
CW15518	4.20	16.140	12.042	14.526	10.838	13.206	9.853	11.342	9.031	9.664	8.337	8.333	7.741		
CW17015	3.67	13.544	11.256	12.190	10.130	11.082	9.209	10.158	8.442	9.377	7.792	8.636	7.236	7.523	6.753
CW17018	4.41	17.575	13.716	15.818	12.344	14.380	11.222	13.181	10.287	11.974	9.496	10.325	8.817	8.994	8.230
CW18515	3.85	15.634	12.297	14.070	11.067	12.791	10.061	11.725	9.223	10.823	8.513	10.050	7.905	9.160	7.387
CW18518	4.62	19.864	15.025	17.878	13.523	16.253	12.294	14.898	11.269	13.752	10.402	12.577	9.659	10.956	9.015
CW20015	4.03			14.866	12.041	13.514	10.946	12.388	10.034	11.435	9.262	10.618	8.600	9.910	8.027
CW20016	4.30			16.373	12.957	14.884	11.779	13.644	10.797	12.594	9.967	11.695	9.255	10.915	8.638
CW20018	4.83			19.369	14.745	17.608	13.405	16.141	12.288	14.899	11.343	13.835	10.532	12.913	9.830
CW20020	5.37			21.378	16.436	19.434	14.942	17.815	13.697	16.444	12.643	15.270	11.740	14.252	10.957

This system is employed where continuity across the supports either can not be provided, i.e. rails are

'CW' Butted Rail - Working Load Capacities in kN

tion No. wt kg/m		4 metre	. 5 2 span	5 metre	.0 Soan	5 metre	.5 Span	6. metre	.0 Span	6. metre	.5 span	7 metre	.0 Span	7.5 metre span	
Section	Self wi	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction	Pressure	Suction
CW12715	3.17	5.468	5.468	4.429	4.429										
CW12716	3.38	5.824	5.824	4.717	4.717										
CW12718	3.80	6.527	6.527	5.287	5.287	4.369	4.369								
CW12720	4.22	7.219	7.219	5.847	5.847	4.832	4.832								
CW14015	3.32	6.835	6.439	5.536	5.536	4.575	4.575	3.844	3.844						
CW14016	3.54	7.281	7.101	5.897	5.897	4.874	4.874	4.095	4.095						
CW14018	3.98	8.163	8.163	6.612	6.612	5.464	5.464	4.591	4.591						
CW15515	3.50	8.639	7.206	6.998	6.486	5.783	5.783	4.860	4.860	4.141	4.141				
CW15518	4.20	10.323	9.384	8.362	8.362	6.911	6.911	5.807	5.807	4.948	4.948				
CW17015	3.67	9.958	7.966	8.666	7.170	7.162	6.518	6.018	5.975	5.128	5.128	4.422	4.422		
CW17018	4.41	12.792	10.404	10.361	9.363	8.563	8.512	7.195	7.195	6.131	6.131	5.286	5.286		
CW18515	3.85	10.896	8.717	9.806	7.845	8.721	7.132	7.328	6.538	6.244	6.035	5.384	5.384	4.690	4.690
CW18518	4.62	14.275	11.420	12.621	10.278	10.431	9.344	8.765	8.565	7.468	7.468	6.439	6.439	5.609	5.609
CW20015	4.03	11.817	9.454	10.635	8.508	9.668	7.735	8.796	7.090	7.495	6.545	6.463	6.077	5.630	5.630
CW20016	4.30	13.077	10.462	11.770	9.416	10.700	8.560	9.376	7.846	7.989	7.243	6.888	6.726	6.001	6.001
CW20018	4.83	15.539	12.431	13.985	11.188	12.525	10.171	10.525	9.323	8.968	8.606	7.732	7.732	6.736	6.736
CW20020	5.37	17.865	14.292	16.078	12.863	13.876	11.693	11.660	10.719	9.935	9.894	8.566	8.566	7.462	7.462

General notes to all 'CW' rail systems

Working load capacities for wind pressure are printed on a dark yellow background. These values are derived from Ultimate Load Capacities divided by 1.4.

Working load capacities for wind suction are printed on a light grey background. These values are derived from Ultimate Load Capacities divided by 1.4.

'CW' Sleeve Rails

All rails must be sleeved across the penultimate support (see page 5 for alternative joint arrangement diagram).

'CW' Butted Rails

within the depth of the column, or is not required to achieve the necessary load carrying capacity (see page 5 for alternative joint arrangement diagram).

The tabulated values will give deflections not exceeding span/150. The load capacities assume that the rails support a screw fixed, steel cladding system. Reference must be made to page 46 for information on bracing requirements and limitations. For applications where any other type of cladding system is 64 - 6685411 employed, please consult Tegral Technical Services Department - 2543499 Phone: 021 - 4551000 Fax: 064 - 6685596

'C' & 'CW' Sections Detailing

'CW' Section Detailing

Sleeved System

100

Typical interior bay sleeved rail for spans over 6.1m up to 9.0m.

Firescreen Rails

100

47

The sleeved System comprises a single bay rail with

14mm dia strut hole

¢ colum

47

Typical interior bay sleeved rail for spans up to 6.1m.

a sleeve connection at alternate supports.

Window and Door Trimmers

'CW' sections can be incorporated within the cladding rail system to act as window trimmers, and also access door frameworks. The formulae given under the "Brickwork Supports" heading may be used to calculate an appropriate window trimmer section.

INSET CLEAT (By special order)										
SECTION RANGE	127-140	155	170-200							
Cleat Code	I1	18	19							
L1 (mm)	130	130	140							
L3 (mm)	90	110	130							
S2 (mm)	42	61	80							
S4 (mm)	105	115	115							
d1 (mm)	14	14	14							
d2 (mm)	13	14	14							

L3

55

Rails should generally be single span, but may be sleeved to achieve continuity.

Note: Sleeves on every joint at penultimate column.

47

a 25 100 100 _____25 25 g 25 14mm dia strut holes Typical interior bay sleeved rail for spans up to 6.1m. Typical interior bay sleeved rail for spans over 6.1m up to 7.5m.

RAIL/CLEAT CONNECTION DETAIL

All fixing holes are 14 dia for M12 grade 8.8 bolts.

General Detailing Notes

Rails may be fixed with the sheeting flange pointing up or down to suit the detail. All sleeves are 2.5mm thick. See page 46 for bracing requirements/details and page 48 for cleat details.

STANDARD DIMENSIONS FOR 'CW' SECTIONS												
Rail depth	d	127	140	155	170	185	200	220	240			
Flange widths	а	65	65	65	65	65	65	65	65			
	b	65	65	65	65	65	65	65	65			
Hole centres	С	42	47	45	43	51	58	70	80			
	е	42	42	61	80	80	80	80	80			
	f	43	51	49	47	54	62	70	80			
Sleeve	g	208	208	208	208	278	278	-	-			
Dimensions	h	566	566	566	566	706	706	-	-			

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d

Sheeting line depth using standard cleats equals section depth +7mm.

(for 127, 140, 155, 170, 185

61

Bracing Strut holes are 14 sq. punched in pairs.

+	+	

For max cantileve projection consult rechnical Services

GABLE END DETAIL

'CW' Section Applications

Brickwork Supports

'CW' sections are increasingly being used as restraint members at the head of masonry walls. These members are often single spanning, and generally deflection critical as a result of tighter permissible limits required when supporting masonry.

The following formulae may be used to determine a suitable section size for a single span, non-continuous rail. It is assumed that the applied load is uniformly distributed, i.e. for a vertical spanning masonry panel:

Deflection:	lxx req	=	dWL ²						
			157.44						
Strength:	Mc req	=	WL						
			5.714						
where:	W	=	Total unfactored applied wind load (kN)						
	L	=	Span (m)						
	d	=	Deflection ratio (i.e. for an $L/360$						
			deflection limit, d = 360)						
lxx req	=	val	lue of Ixx required (cm4)						
Mc req	=	val	lue of moment capacity required (kNm)						
	Youngs	mo	dulus in deflection equation taken as 205						
	kN/mm ²								

There are a number of suitable methods available for tying the masonry to the 'CW' section, these include:

- 1. proprietary sliding anchor ties bolted to the 'CW' section
- 2. threaded rod with standard ties welded to the underside and secured to the 'CW' section with a nut top and bottom
- flat sheet strips fixed to 3. the 'CW' section using self drilling, self tapping screws, bent down into the perpend joints of the masonry during construction.

Panel Joint Rail Sections

Panel Joint Rail Sections (PJR)

	Specification	
Ø	Section	Grade S390GD+Z275, zinc coated steel with a minimum
		yield strength of 390 N/mm ² .
		Refer to page 80 for full
		technical specification
Ø	Design	BS5950: Part 5: 1998

STANDARD DIMENSIONS FOR ZED SECTIONS										
Purlin depth	d	125	140	155	170	185	200			
Flange widths	а	55	58	58	58	58	58			
	b	49	49	49	49	49	49			
Hole centres	С	40	47	45	43	51	58			
	е	42	42	61	80	80	80			
	f	43	51	49	47	54	63			
Sleeve	g	208	208	208	208	278	278			
Dimensions	h	566	566	566	566	706	706			

Section	Depth	Ą	t	Weight	Area	lxx	ZXX	LXX	lyy	ZYY	гуу	×	٨	Mca	Mcb	
	mm	mm	mm	kg/m	mm ²	cm ⁴	cm ³	mm	cm ⁴	cm ³	mm	mm	mm	kNm	kNm	
PJR140	140	1.8	47	5.67	702	208.14	22.97	54.51	225.70	20.00	56.76	52.09	49.32	8.78	8.09	
PJR155	155	1.8	52	5.95	737	272.56	27.70	60.87	232.26	20.30	56.19	50.55	56.55	10.04	9.52	
PJR170	170	1.8	57	6.23	772	347.80	32.82	67.19	238.74	20.63	55.66	49.20	63.98	11.31	10.98	
PJR185	185	1.8	62	6.51	807	434.47	38.32	73.44	245.29	20.98	55.18	48.02	71.58	12.60	12.43	
PJR200	200	1.8	47	6.51	807	480.78	38.21	77.26	249.66	20.89	55.67	45.42	74.12	13.36	12.12	

' M_{α} ' refers to the Ultimate Restrained Capacity of the section with the wide (sheeted) flange in compression. ' M_{α} ' refers to the Ultimate Restrained Capacity of the section with the narrow (unsheeted) flange in compression. For suction loading, it is suggested that the stress ratio is restricted to 0.80, i.e. M_x/M_{\odot} <0.80.

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Tegral Panel Joint Rails (PJRs) have been introduced to offer a single, wide flanged member to support horizontal cladding panels at joint positions, where minimum bearing width stipulations would otherwise require the use of a compound solution.

The sections are available from 140mm to 200mm in depth, and are all a minimum 1.8mm thick to ensure that the flange is sufficiently stiff to allow the sheeters to fix the cladding panels.

The sections are designed to interconnect with the similar depth Zed and CW sections using the Inset cleats shown on page 59, and provide a flush external and internal line.

Standard holing lines for all sections

14 dia. web holes may be punched in any pattern on these lines (Dimensions f and c match those shown on page 39 for the Zed range).

Section Properties

Top Hat Sections

Section Properties

	Section	Depth	بو mm	D dea	ka/m	Prea mm ²	XX cm ⁴	XXZ	X mm	K I cm⁴	λλ cm³	Š	≻ mm	e ow kNm	q Wm
-	TH80	80	1.7	70	4.15	508	50.73	11.61	31.26	159.74	15.36	55.46	43.76	4.28	3.60
1	THPJ80	80	1.8	70	5.65	702	77.48	17.09	32.93	551.99	36.56	87.88	34.66	4.80	4.43
1	TH100	100	1.7	73	4.61	571	83.96	15.58	38.13	172.9	16.63	54.71	53.96	5.74	4.96
1	THPJ100	100	1.8	73	6.25	772	127.25	22.67	40.39	612.38	40.29	88.61	43.86	6.52	6.06

 M_{ca} refers to the Ultimate Restrained Capacity of the section with the stiffened (sheeted) flange in compression. M_{cb} refers to the Ultimate Restrained Capacity of the section with the two non-stiffened flange in compression.

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Zeta 175 rails illustrated

DETAIL 1 Sleeve detail. DETAIL 2 Rail and cleat assembly at diagonal brace position.

Eaves Tie

Purlins - Construction Details

Cleat Zeta 200 purlins illustrated DETAIL 1 Apex Tie Quick-lok Sag Bar (where required) DETAIL Gable Rail Vertical Strut Cleader Member (extended to eaves and apex)

6

DETAIL 3 Corner column cleat detail using extended cleat.

Purlins - Construction Details

Monopitched Roofs

Where sag bars are required for a conventional duo-pitched roof, in accordance with the tables on pages 14, 29 or 41 they will also be required for a monopitched roof.

It is suggested that strut and diagonal bracing sections are provided between the top two purlins at the higher eaves. This acts as a substitute for the apex tie member in a conventional duo-pitched roof and ties the purlin system back to the steelwork. Sag bars can be provided in the required manner below these top two purlins.

Purlins - Construction Details

Cantilever Purlins

It is sometimes necessary to extend purlins and cladding rails beyond the gable frame creating a cantilever condition. In this situation, the following points should be considered:

- 1. The sections forming the cantilever must be continuous across the gable frame. The stiffness of the cantilever is improved if continuity is also provided across the penultimate support (eg by providing a sleeve).
- 2. The cantilever ends of the sections must be braced to prevent lateral displacement. The Ayrshire strut section indicated on pages 21, 33 or 47 can be provided as a tie member. Alternatively, where the cantilevered sections are to be clad, the inclusion of cleader members to support the fascia cladding should be sufficient.
- 3. For steep slopes i.e. $\theta > 20^{\circ}$ for Zeta, θ > 30° for Zeta II and Zed, diagonal members should be located at the top of the slope (or at the base for cladding rails) to prevent displacement.
- 4. Cantilevers are generally deflection rather than strength critical. We suggest that deflection of the cantilever is checked for the worst relevent load combination before confirming its strength capacity. For further advice and confirmation of suitable sizes for given condition, please consult our Technical Services Department.

Zeta 2 and Zed sections (strut type VS1)

Alternate lines reversed with strut type bracing between Note: for all Zeta II and Zed sections provide Type VS1 struts Now available Patrick Lynch Roof Cladding

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Shallow Pitched Roofs ($\theta < 3^{\circ}$)

In this situation it is necessary to avoid a potential buildup of lateral force in any one direction due to the inclination of the principle axes of the purlins. This can be achieved by reversing alternate lines of section to face each other. When sag bars are required in accordance with the tables on pages 14, 29 or 41 we recommend that pairs of sections are braced together at the appropriate positions (i.e. mid-span or third points) using a standard Tegral strut section, VS1 or VS2.

Struts fixed in bottom holes

An alternative solution is to provide an "eaves tie" member at the high eaves position, fixed back to a suitable structural member capable of carrying the downslope forces from the purlins. It should be noted that where sag bars are not

required in accordance with the tables on pages 14, 29 or 41 (i.e. Zeta purlins in the majority of applications), no sag bars are required to the equivalent monopitched roof.

Purlins - Construction Details

Tiled Roofs

The use of cold rolled steel purlin systems to support traditional tiled roof constructions has become increasingly popular over recent years. The following offers design and installation advice with regard to the purlins in this situation.

Unlike a conventional screw fixed steel cladding system, a built up tiled roof construction is not deemed to act as a diaphragm panel. Consequently, the purlin sections must be designed for a bi-axial bending condition, taking into account the downslope forces. The timber rafters fixed to the top flange of the purlins are still considered to provide full lateral restraint to that flange in compression. Struts are provided to act as supports to the downslope forces as appropriate, with diagonal braces to transfer these forces back to the supporting steelwork. It should be noted that ties across the apex are not required in this situation, assuming the timber rafters are connected at the ridge point.

The number of struts and diagonal bracings required is a function of the geometry and loading conditions in each individual case, and advice should be sought from Tegral Technical Services Department. Alternatively, the Tegral Purlin and Cladding Rail Design Software providing an accurate and economic design, is available upon request.

Tiled Roof Cleats

Due to the increase in Dead loads for a tiled roof construction, coupled with a steeper roof slope, a check on the suitability of Tegrals standard cleats (shown on pages 22, 34 or 48) must be made, and a stiffener plate

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provided where appropriate. Particular care must be taken where the diagonal braces are secured back to the steelwork. Due to the magnitude of force involved, it is suggested that these are fixed directly to the steelwork flange.

appropriate screw fixing.

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Angle cleat fixed with appropriate screw fixing (by others). Timber rafter fixed direct to putin using

Ductcover System

Tegral has developed a Ductcover system to complement our purlin ranges. The system is manufactured from lightweight white polyester coated galvanised steel as a standard finish.

The range of components available are designed to offer a comprehensive system compatible with standard electrical accessories and service suspension clips and hangers.

Purlins - Construction Details

System Components

Standard Ductcover strip, 3m length

Optional 1m length Ductcover strip

Standard Ductcover Joint Strap

Gland Inlet/Outlet Joint Strap supplied with a 20mm dia hole and a 15A 3 No. Terminal Strip (supplied with or without gland)

Notched Ductcover Joint Strap for use at service suspension hanger positions

Standard single PVC trunking box (25mm depth) with front cover strip to suit MK Crabtree or Tenby socket/spur cover

Earth Continuity Strip

DESIGNER'S STRUCTURAL PRODUCTS GUIDE

Purlins - Construction Details

Purlins - Construction Details

Duct cover Installation

 Drop the Trunking Box into the bottom flange of the purlin at the required position. Secure the box to the web of the purlin at this level using self drilling, self tapping screws or bolts as appropriate.

2. Snap the Standard Ductcover Strips into place and slide along to abut the Trunking Box. They are designed to lap over the outstand of the Trunking Box cover strip. Optional 1m lengths of the Standard Ductcover Strip are also available. For Zeta 125 and 200 deep sections, together with all the Zeta 2, their use is suggested at cleat positions where the strips must be snapped into place clear of the bolt positions and slid along under the bolt heads as appropriate.

- 3. Snap over the Standard Joint Straps between strips. A nominal clearance between abutting strips of 50mm is suggested.
- 4. Notched Joint Straps are located over services suspension clips and hangers where appropriate.
- 5. Gland Inlet/Outlet Pieces are treated in a similar manner, although we suggest that the gap between the Ductcover strips should be 100mm minimum to allow for the gland and terminal strip.
- Earth Continuity Strips are available for all non continuous joints in the purlin system, i.e. unsleeved joints. These can be located over the lip to the bottom flange of the purlin.
 A Notched Joint Strap can be placed over the Earth Continuity Strip as appropriate.

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Service Suspension Clips and Hangers

A proprietary range of clips and hangers for supporting services are available in the market to suit all Tegral's purlin sections (see below). The effect of an applied point load on the purlin must be checked. We suggest that these loads should be recalculated as an equivalent udl applied to a simply supported beam using the formula below:

		L	a, b, L in metres
he equivalent udl	=	8M	(in kN)

The capacities of the various clips and hangers themselves are available from the manufacturers. It should be noted, however, that for clips supported off the bottom flange of the purlin only, the maximum applied loading should not exceed 15kg (working load) to ensure that lateral displacement of the purlin does not occur. This is irrespective of any higher capacities quoted by the clip manufacturers. Where the loading exceeds 15kg, this load must be transferred back to the web, or whole, of the section using the appropriate part wrap-around or total wrap-around bracket.

Purlins - Construction Details

Parapet Details

Tegral's comprehensive range of sections can be integrated to form a parapet detail as required. The rails above eaves level are often single

spanning between the hot rolled cantilevered posts at each frame position. A 'C' or Eaves Beam section may be incorporated as the top, capping member to suit the detail.

The standard strut and diagonal bracing system should be continued up to and including this top rail.

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The eaves tie detail indicated on pages 14, 29 or 41 is not deemed to provide a lateral restraint position in this situation, and we would recommend the use of one of the alternative details shown above.

Restraint Struts

Where cladding is deemed not to provide lateral restraint, special restraining sections, termed "Restraint Struts", are available to act as minor axis supports against lateral buckling as appropriate. For shallow pitched roof applications ($\theta < 3^{\circ}$), two sets of diametrically opposed diagonal bracings should be provided, as a minimum, to transfer the restraint forces back to the supporting structure. For steeper slopes, one set at eaves level should be provided as a minimum. The total number of diagonal bracings required, in one or both directions, is a function of the roof slope length.

50 x 50 angle, length L to suit standard holing line centres for each Zeta II and Zed section plus 50mm. (Clamp plate length amended to suit). For Zeta 125-175 range, use standard VS1 strut. For Zeta 200 range use standard VS2 strut, (see page 20).

Now available from Patrick Lynch Roof Cladding info@patricklynchroofcladding.com www.patricklynchroofcladding.com Non-Restraining Cladding

Certain types of sheeting are not deemed to provide lateral restraint to the purlin and cladding rail sections. These include clip fixed standing seam profiles fixed directly to the sections, without the presence of a steel lining panel, and certain secret fix wall cladding panels. Another common application where this situation arises would be for purlins supporting a proprietary suspended ceiling system.

The load tables published in this manual are not applicable in these situations, and advice should be sought from Tegral Technical Services Department. Alternatively, the Tegral Purlin and Cladding Rail Design Software provides an accurate and economic design, and is available upon request.

VERTICAL AND HORIZONTAL GRILLAGE

Horizontal Cladding

Where horizontal cladding is required to be fixed to the cladding rail system, there are a number of alternative solutions available to the designer. The following pages highlight four possible methods.

Vertical spanning rails 1.

Vertical spanning rails between a structural top and bottom member, and across any intermediate members as appropriate. In this solution, "ties" between pairs of sections should be provided to prevent lateral displacement under loading.

2. Mini-zed spacers in the cavity of a built-up cladding system

If a built-up cladding system is employed, the liner panel can be fixed vertically in the conventional manner between horizontal rails, with mini-zed spacer bars spanning vertically between rails. The horizontal outer sheet can then be fixed to these spacer bars. This solution assumes that the vertical liner is aesthetically acceptable, and the mini-zed spacer bar is structurally capable of spanning between the horizontal rails. It should also be noted that, in this situation, the horizontal rails are restrained laterally and the published load tables in this manual are applicable.

Rails - Construction Details

4. Structural Top Hat solution

A similar solution to the vertical rail/PJR arrangement shown on the previous pages, but using fewer components and fixings.

The use of a Top Hat section spanning between the conventional horizontal rails, and fixed to the outer flange removes the need for Inset cleats and reduces the number of bolt fixings required by up to 20%.

A separate bracing support system (Restraint Struts and Diagonal Bracing) is required for the horizontal rails to afford restraint and maintain alignment during construction. A "Panel Joint Top Hat" is also available for cladding joint positions - see details below.

Rails - Construction Details

3. Vertical and horizontal cold rolled grillage The most frequently employed detail involves the use of vertical spanning rails fixed between the conventional horizontal spanning cladding rail system as illustrated opposite.

Zed sections can be used for the vertical rails, with "Panel Joint Rails" (PJRs) at cladding joint positions where a double bearing width is required. Tegral's PJR range is fully compatible with the Zed sections and can be interconnected using the appropriate Inset cleat. Refer to page 61 for PJR properties, and page 59 for Inset cleat details.

The PJR section has been specifically developed such that the centre line of the flange coincides with the gridline when standard Inset cleats are used. The vertical rails act as "struts" in the conventional strut and diagonal brace system required for the horizontal rails. It should be noted that the horizontal rails in this situation are laterally restrained at cleat and vertical rail positions only, and should be checked for lateral buckling between these points accordingly. Please consult Tegral Technical Services Department for advice. Alternatively, the "Tegral" Purlin and Cladding Rail Design Software can be used to design laterally unrestrained sections. This software is available upon request.

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The Top Hat profile can be connected to the full range of Tegral sections, primarily Zeta, Zeta 2, Zed and CW ranges.

It should be noted that the horizontal rails in this situation are not restrained by the cladding system, and a separate restraining system in the form of Restraint Struts is required. Please consult Tegral Technical Services Department for advice. Alternatively, the "Tegral" Purlin and Cladding Rail Design Software can be used to design laterally unrestrained sections. This Software is available upon request.

Slotted holes are available in the vertical Top Hat sections, and horizontally in the flange of the rails. This provides vertical and horizontal adjustment on site during construction to aid alignment.

Zeta Ultimate Load Capacities

Ultimate Load Capacities

Zeta Double Span (Purlin & Rail) - Ultimate Load Capacities in kN

Span (m)	Section No.	1/12513	1/12514	1/12515	1/12517	1/12520	1/15013	1/15014	1/15015	1/15017	1/15020	1/17513	1/17514	1/17515	1/17517	1/17520	1/20013	1/20014	1/20015	1/20016	1/20018	1/20020	1/20025
	Self Weight	2.73	2.95	3.16	3.58	4.21	3.28	3.53	3.78	4.28	5.04	3.53	3.80	4.07	4.62	5.43	3.79	4.08	4.37	4.66	5.24	5.82	7.22
4.50	Gravity Load	12.658	13.856	15.039	17.380	20.187	17.157	19.073	20.921	24. 523	29.175	20.188	22.510	24.751	29.135	34.862	22.999	25.746	28.398	31.015	36.161	40.468	52.612
	δ = Span/180	8.252	8.883	9.515	10.778	12.634	14.881	16.031	17.182	19.474	22.866	20.193	21.770	23.329	26.446	31.073	28.678	30.921	33.155	35.378	39.796	44.193	55.033
	Wind Uplift	11.075	12.124	13.159	15.207	17.663	15.013	16.689	18.306	21.457	25.528	17.664	19.697	21.657	25.493	30.504	20.124	22.528	24.848	27.138	31.641	35.409	46.036
5.00	Gravity Load	11.392	12.470	13.535	15.642	18.168	15.442	17.166	18.829	22.070	26.258	18.169	20.259	22.276	26.222	31.375	20.699	23.171	25.558	27.913	32.545	36.421	47.351
	δ = Span/180	6.525	7.024	7.524	8.523	9.990	12.053	12.985	13.918	15.774	18.521	16.356	17.634	18.896	21.421	25.169	23.229	25.046	26.855	28.657	32.235	35.797	44.577
	Wind Uplift	9.968	10.912	11.843	13.687	15.897	13.511	15.020	16.476	19.312	22.975	15.898	17.727	19.491	22.944	27.454	18.112	20.275	22.363	24.424	28.477	31.868	41.432
5.50	Gravity Load	10.356	11.337	12.305	14.220	16.516	14.038	15.605	17.118	20.064	23.871	16.517	18.418	20.251	23.838	28.523	18.817	21.065	23.235	25.376	29.586	33.110	43.046
	δ = Span/180	5.521	5.944	6.366	7.212	8.453	9.961	10.732	11.502	13.036	15.307	13.518	14.574	15.617	17.703	20.801	19.198	20.699	22.195	23.683	26.640	29.584	36.840
	Wind Uplift	9.062	9.920	10.767	12.442	14.452	12.283	13.655	14.978	17.556	20.887	14.452	16.115	17.719	20.858	24.958	16.465	18.432	20.330	22.204	25.888	28.971	37.666
6.00	Gravity Load	9.493	10.392	11.279	13.035	15.140	12.868	14.305	15.691	18.392	21.881	15.141	16.883	18.563	21.851	26.146	17.249	19.309	21.298	23.261	27.121	30.351	39.459
	$\delta = \text{Span}/180$	4.486	4.829	5.172	5.859	6.868	8.370	9.018	9.665	10.954	12.862	11.359	12.246	13.123	14.876	17.479	16.131	17.393	18.650	19.900	22.385	24.859	30.956
	Wind Uplift	8.307	9.093	9.869	11.405	13.247	11.259	12.517	13.730	16.093	19.146	13.248	14.772	16.243	19.120	22.878	15.093	16.896	18.636	20.353	23.731	26.557	34.527
6.50	Gravity Load	8.763	9.593	10.412	12.032	13.975	11.878	13.205	14.484	16.977	20.198	13.976	15.584	17.135	20.171	24.135	15.922	17.824	19.660	21.472	25.035	28.016	36.424
	$\delta = \text{Span}/180$	3.874	4.171	4.467	5.060	5.932	6.883	7.415	7.948	9.008	10.577	9.678	10.434	11.181	12.675	14.893	13.745	14.820	15.891	16.957	19.074	21.181	26.377
	Wind Uplift	7.668	8.393	9.110	10.528	12.228	10.393	11.554	12.674	14.855	17.673	12.229	13.636	14.993	17.649	21.118	13.932	15.596	17.203	18.788	21.905	24.514	31.871
7.00	Gravity Load						11.030	12.261	13.449	15.765	18.755	12.978	14.471	15.911	18.730	22.411	14.785	16.551	18.256	19.938	23.246	26.015	33.822
	$\delta = \text{Span}/180$						5.655	6.092	6.529	7.400	8.689	8.308	8.957	9.599	10.881	12.785	11.852	12.779	13.702	14.621	16.446	18.264	22.743
	Wind Uplift						9.651	10.729	11.768	13.794	16.411	11.355	12.662	13.922	16.389	19.610	12.937	14.482	15.974	17.446	20.341	22.763	29.594
7.50	Gravity Load									14.714	17.505	12.113	13.506	14.850	17.481	20.917	13.799	15.447	17.039	18.609	21.697	24.281	31.567
	$\delta = \text{Span}/180$									7.011	8.232	7.167	7.727	8.280	9.386	11.028	10.324	11.132	11.936	12.736	14.326	15.910	19.812
	Wind Uplift									12.874	15.317	10.598	11.818	12.994	15.296	18.302	12.075	13.517	14.909	16.283	18.985	21.245	27.621

Zeta Heavy End Bay (Purlin & Rail) - Ultimate Load Capacities in kN

				<u> </u>			·						
(ui) ueds	Section No.	1/15013 .17	1/15014 .20	1/15015 .20	1/17513 .17	1/17514 .20	1/17515 .20	1/20013 .18	1/20014 .18	1/20014 .20	1/20015 .20	1/20016 .25	1/20018 .25
	Swt. (End Bay) 4.28	5.04	5.04	4.62	5.43	5.43	5.24	5.24	5.82	5.82	7.22	7.22
4.50	Gravity Load	23.112	25.693	28.183	27.194	30.324	33.342	30.982	34.682	34.682	38.255	40.598	48.627
	δ = Span/180	18.877	22.165	22.165	24.112	28.332	28.332	37.879	37.879	42.065	42.065	52.383	52.383
	Wind Uplift	19.669	22.482	24.660	22.571	25.169	29.174	25.715	28.786	28.786	33.473	35.524	42.548
5.00	Gravity Load	20.801	23.124	25.365	24.475	27.291	30.007	27.884	31.214	31.214	34.429	36.539	43.764
	δ = Span/180	15.291	17.954	17.954	19.531	22.949	22.949	30.682	30.682	34.073	34.073	42.430	42.430
	Wind Uplift	17.702	20.234	22.194	20.314	22.652	26.257	23.143	25.907	25.907	30.126	31.971	38.294
5.50	Gravity Load	18.910	21.022	23.059	22.250	24.810	27.279	25.349	28.376	28.376	31.299	33.217	39.786
	δ = Span/180	12.637	14.838	14.838	16.141	18.966	18.966	25.357	25.357	28.159	28.159	35.066	35.066
	Wind Uplift	16.093	18.394	20.177	18.468	20.592	23.870	21.039	23.552	23.552	27.387	29.065	34.812
6.00	Gravity Load	17.334	19.270	21.137	20.396	22.743	25.006	23.236	26.011	26.011	28.691	30.449	36.470
	δ = Span/180	10.618	12.468	12.468	13.563	15.937	15.937	21.307	21.307	23.662	23.662	29.465	29.465
	Wind Uplift	14.751	16.861	18.495	16.929	18.876	21.880	19.286	21.589	21.589	25.105	26.643	31.911
6.50	Gravity Load	16.001	17.788	19.511	18.827	20.993	23.083	21.449	24.011	24.011	26.484	28.107	33.665
	$\delta = \text{Span}/180$	8.396	9.858	9.858	11.557	13.579	13.579	18.155	18.155	20.161	20.161	25.107	25.107
	Wind Uplift	13.617	15.564	17.072	15.626	17.424	20.197	17.803	19.929	19.929	23.173	24.593	29.457
7.00	Gravity Load	14.858	16.517	18.118	17.482	19.494	21.434	19.917	22.296	22.296	24.592	26.099	31.260
	$\delta = \text{Span}/180$	6.505	7.638	7.638	9.868	11.594	11.594	15.654	15.654	17.384	17.384	21.648	21.648
	Wind Uplift	12.644	14.453	15.853	14.510	16.180	18.755	16.531	18.505	18.505	21.518	22.837	27.353
7.50	Gravity Load				16.317	18.194	20.005	18.589	20.809	20.809	22.953	24.359	29.176
	$\delta = \text{Span}/180$				8.412	9.884	9.884	13.637	13.637	15.143	15.143	18.858	18.858
	Wind Uplift				13.543	15.101	17.504	15.429	17.272	17.272	20.084	21.314	25.529
8.00	Gravity Load				15.297	17.057	18.755	17.427	19.509	19.509	21.518	22.837	27.353
	$\delta = \text{Span}/180$				/.137	8.386	8.386	11.985	11.985	13.310	13.310	16.574	16.574
	Wind Uplift				12.696	14.15/	16.410	14.465	16.192	16.192	18.828	19.982	23.933
8.50	Gravity Load							16.402	18.361	10.072	20.252	21.493	25./44
	0 = Span/180 Wind Unlift							9.880	9.880	10.972	10.972	15.664	13.664
Root Cladding								13.014	15.240	15.240	17.721	10.607	22.520
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Span (m)	Section No.	1/12513	1/12514	1/12515	1/12517	1/12520	1/15013	1/15014	1/15015	1/15017	1/15020	1/17513	1/17514	1/17515	1/17517	1/17520	1/20013	1/20014	1/20015	1/20016	1/20018	1/20020	1/20025
	Self Weight	2.73	2.95	3.16	3.58	4.21	3.28	3.53	3.78	4.28	5.04	3.53	3.80	4.07	4.62	5.43	3.79	4.08	4.37	4.66	5.24	5.82	7.22
4.00	Gravity Load	11.651	12.754	13.843	15.997	18.925	14.038	15.605	17.118	20.064	24.313	16.517	18.418	20.251	23.838	29.051	18.817	21.065	23.235	25.376	29.586	33.723	43.843
	δ = Span/180	5.534	5.958	6.381	7.229	8.473	8.813	9.495	10.177	11.534	13.543	11.546	12.448	13.339	15.121	17.767	15.793	17.029	18.259	19.483	21.916	24.338	30.307
	Wind Uplift	9.554	10.458	11.351	13.118	15.518	11.511	12.796	14.036	16.453	19.936	13.544	15.102	16.606	19.547	23.822	15.430	17.273	19.052	20.808	24.261	27.653	35.952
4.50	Gravity Load	10.356	11.337	12.305	14.220	16.822	12.478	13.871	15.216	17.835	21.611	14.682	16.371	18.001	21.189	25.823	16.727	18.724	20.653	22.556	26.299	29.976	38.972
	δ = Span/180	4.373	4.707	5.042	5.712	6.695	6.964	7.502	8.041	9.113	10.700	9.122	9.835	10.539	11.947	14.038	12.479	13.455	14.427	15.394	17.316	19.230	23.946
	Wind Uplift	8.492	9.296	10.090	11.660	13.794	10.232	11.375	12.477	14.624	17.721	12.039	13.424	14.760	17.375	21.175	13.716	15.354	16.936	18.496	21.565	24.580	31.957
5.00	Gravity Load	9.321	10.203	11.074	12.798	15.140	11.230	12.484	13.694	16.051	19.450	13.214	14.734	16.201	19.070	23.241	15.054	16.852	18.588	20.301	23.669	26.978	35.075
	δ = Span/180	3.542	3.813	4.084	4.626	5.423	5.641	6.077	6.513	7.382	8.667	7.389	7.966	8.537	9.677	11.371	10.108	10.898	11.686	12.469	14.026	15.576	19.397
	Wind Uplift	7.643	8.366	9.081	10.494	12.415	9.209	10.237	11.229	13.162	15.949	10.835	12.082	13.284	15.638	19.058	12.344	13.818	15.242	16.647	19.409	22.122	28.761
5.50	Gravity Load	8.473	9.275	10.068	11.634	13.764	10.209	11.349	12.449	14.592	17.682	12.012	13.395	14.728	17.337	21.128	13.685	15.320	16.898	18.455	21.517	24.526	31.886
	$\delta = \text{Span}/180$	2.927	3.151	3.375	3.824	4.482	4.662	5.022	5.383	6.101	7.163	6.107	6.584	7.055	7.998	9.397	8.353	9.007	9.657	10.305	11.592	12.873	16.030
	Wind Uplift	6.948	7.606	8.255	9.540	11.286	8.372	9.306	10.208	11.965	14.499	9.850	10.984	12.077	14.216	17.325	11.222	12.562	13.856	15.133	17.644	20.111	26.147
6.00	Gravity Load	7.767	8.503	9.229	10.665	12.617	9.358	10.404	11.412	13.376	16.208	11.011	12.278	13.500	15.892	19.368	12.545	14.043	15.490	16.917	19.724	22.482	29.229
	$\delta = \text{Span}/180$	2.460	2.648	2.836	3.213	3.766	3.917	4.220	4.523	5.126	6.019	5.131	5.532	5.928	6.720	7.896	7.019	7.568	8.115	8.659	9.740	10.817	13.470
	Wind Uplift	6.369	6.972	7.567	8.745	10.346	7.674	8.531	9.358	10.968	13.291	9.029	10.068	11.070	13.031	15.881	10.287	11.515	12.702	13.872	16.174	18.435	23.968
6.50	Gravity Load						8.639	9.603	10.534	12.347	14.962	10.164	11.334	12.462	14.669	17.878	11.580	12.963	14.298	15.616	18.207	20.753	26.981
	= Span/180						3.338	3.596	3.854	4.368	5.129	4.372	4.714	5.051	5.726	6.728	5.981	6.449	6.915	7.378	8.299	9.217	11.477
	Wind Uplift						7.084	7.875	8.638	10.125	12.269	8.335	9.294	10.219	12.029	14.660	9.496	10.630	11.725	12.805	14.930	17.017	22.124
7.00	Gravity Load											9.438	10.524	11.572	13.622	16.601	10.753	12.037	13.277	14.500	16.906	19.270	25.053
	$\delta = \text{Span}/180$											3.770	4.065	4.355	4.937	5.801	5.15/	5.560	5.962	6.362	7.156	7.947	9.896
	Wind Uplift											1./39	8.630	9.489	11.1/0	13.613	8.81/	9.8/0	10.887	11.890	13.863	15.802	20.544
7.50	Gravity Load											8.809	9.823	10.800	12.714	15.494	10.036	11.235	12.392	13.534	15.779	17.986	23.383
	o = Span/180											3.284	3.541	3.794	4.301	5.054	4.492	4.844	5.194	5.542	6.234	6.923	8.621
	wind Uplift											1.223	8.055	8.856	10.425	12.705	8.229	9.212	10.161	11.098	12.939	14./48	19.1/4

Zeta Heavy End Bay (Purlin & Rail) - Ultimate Load Capacities in kN

an (m)	Section No.	1/15013 .17	1/15014 .20	1/15015 .20	1/17513 .17	1/17514 .20	1/17515 .20	1/20013 .18	1/20014 .18	1/20014 .20	1/20015 .20	1/20016 .25	1/20018 .25
Ъ	Swt. (End Bay)	4.28	5.04	5.04	4.62	5.43	5.43	5.24	5.24	5.82	5.82	7.22	7.22
4.50	Gravity Load	22.149	24.959	27.377	26.061	29.457	32.389	29.691	33.691	33.691	37.161	39.438	47.237
	δ = Span/180	18.877	22.165	22.165	24.112	28.332	28.332	37.879	37.879	42.065	42.065	52.383	52.383
	Wind Uplift	18.849	21.839	23.955	21.631	24.449	28.340	24.643	27.963	27.963	32.516	34.508	41.332
5.00	Gravity Load	19.934	22.463	24.640	23.455	26.511	29.150	26.722	30.321	30.321	33.445	35.494	42.513
	δ = Span/180	15.291	17.954	17.954	19.531	22.949	22.949	30.682	30.682	34.073	34.073	42.430	42.430
	Wind Uplift	16.964	19.655	21.560	19.468	22.004	25.506	22.179	25.167	25.167	29.264	31.057	37.199
5.50	Gravity Load	18.122	20.421	22.400	21.323	24.101	26.500	24.293	27.565	27.565	30.405	32.267	38.648
	δ = Span/180	12.637	14.838	14.838	16.141	18.966	18.966	25.357	25.357	28.159	28.159	35.066	35.066
	Wind Uplift	15.422	17.868	19.600	17.698	20.004	23.187	20.163	22.879	22.879	26.604	28.234	33.817
6.00	Gravity Load	16.612	18.719	20.533	19.546	22.093	24.291	22.268	25.268	25.268	27.871	29.578	35.428
	δ = Span/180	10.618	12.468	12.468	13.563	15.937	15.937	21.307	21.307	23.662	23.662	29.465	29.465
	Wind Uplift	14.137	16.379	17.966	16.223	18.337	21.255	18.483	20.972	20.972	24.387	25.881	30.999
6.50	Gravity Load	15.334	17.279	18.954	18.042	20.393	22.423	20.555	23.324	23.324	25.727	27.303	32.702
	δ = Span/180	8.396	9.858	9.858	11.557	13.579	13.579	18.155	18.155	20.161	20.161	25.107	25.107
	Wind Uplift	13.049	15.119	16.584	14.975	16.926	19.620	17.061	19.359	19.359	22.511	23.890	28.615
7.00	Gravity Load	14.239	16.045	17.600	16.754	18.936	20.821	19.087	21.658	21.658	23.889	25.353	30.367
	δ = Span/180	6.505	7.638	7.638	9.868	11.594	11.594	15.654	15.654	17.384	17.384	21.648	21.648
	Wind Uplift	12.117	14.039	15.400	13.906	15.717	18.219	15.842	17.976	17.976	20.903	22.184	26.571
7.50	Gravity Load				15.637	17.674	19.433	17.815	20.214	20.214	22.297	23.663	28.342
	δ = Span/180				8.412	9.884	9.884	13.637	13.637	15.143	15.143	18.858	18.858
	Wind Uplift				12.979	14.669	17.004	14.786	16.778	16.778	19.510	20.705	24.799

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Zeta - Ultimate Load Capacities

Zeta Butted (Purlin & Rail) - Ultimate Load Capacities in kN

Zeta 2 Ultimate Load Capacities

Ultimate Load Capacities

DESIGNER'S STRUCTURAL PRODUCTS GUIDE

Zeta 2 Sleeved (Purlin & Rail) - Ultimate Load Capacities in kN

an (m)	Section No.	2/22514	2/22515	2/22516	2/22518	2/22520	2/22525	2/24514	2/24515	2/24516	2/24518	2/24520	2/24525	2/26515	2/26516	2/26518	2/26520	2/26525	2/26530	2/28515	2/28516	2/28518	2/28520	2/28525	2/28530
ς,	Self Weight	4.47	4.79	5.10	5.73	6.37	7.94	4.69	5.02	5.35	6.02	6.68	8.31	5.26	5.61	6.30	6.99	8.72	10.44	5.49	5.86	6.58	7.31	9.11	10.91
6.00	Gravity Load	19.899	22.407	24.981	30.258	35.719	48.839	21.694	24.489	27.353	33.212	39.263	53.853	26.511	29.674	36.136	42.794	58.910	73.991	26.567	31.900	38.981	46.260	63.949	80.583
	δ = Span/180	19.430	20.842	22.248	25.050	27.836	34.733	24.396	26.171	27.939	31.463	34.968	43.649	32.359	34.547	38.910	43.251	54.006	64.630	39.233	41.890	47.185	52.456	65.522	78.433
	Wind Uplift	15.919	17.926	19.985	24.207	28.575	39.071	17.355	19.591	21.882	26.570	31.410	43.082	21.209	23.739	28.909	34.235	47.128	59.193	22.744	25.520	31.185	37.008	51.159	64.466
6.50	Gravity Load	18.368	20.684	23.060	27.931	32.972	45.082	20.025	22.606	25.249	30.657	36.242	49.710	24.471	27.391	33.356	39.502	54.378	68.299	26.243	29.446	35.982	42.701	59.030	74.384
	δ = Span/180	16.555	17.758	18.957	21.345	23.718	29.595	20.787	22.299	23.806	26.809	29.795	37.192	27.572	29.437	33.154	36.853	46.017	55.070	33.429	35.693	40.205	44.696	55.830	66.831
	Wind Uplift	14.695	16.547	18.448	22.345	26.377	36.065	16.020	18.084	20.199	24.526	28.994	39.768	19.577	21.913	26.685	31.602	43.503	54.639	20.995	23.557	28.786	34.161	47.224	59.507
7.00	Gravity Load	17.056	19.206	21.412	25.936	30.617	41.862	18.595	20.991	23.445	28.468	33.654	46.160	22.724	25.435	30.974	36.680	50.494	63.421	24.369	27.343	33.412	39.651	54.813	69.071
	δ = Span/180	14.275	15.312	16.346	18.404	20.451	25.518	17.924	19.228	20.527	23.116	25.691	32.069	23.774	25.382	28.587	31.776	39.678	47.483	28.824	30.776	34.667	38.539	48.139	57.624
	Wind Uplift	13.645	15.365	17.130	20.748	24.493	33.489	14.876	16.793	18.756	22.774	26.923	36.928	18.179	20.348	24.779	29.344	40.395	50.737	19.495	21.874	26.730	31.721	43.851	55.257
7.50	Gravity Load	15.919	17.926	19.985	24.207	28.575	39.071	17.355	19.591	21.882	26.570	31.410	43.082	21.209	23.739	28.909	34.235	47.128	59.193	22.744	25.520	31.185	37.008	51.159	64.466
	δ = Span/180	12.435	13.339	14.239	16.032	17.815	22.229	15.614	16.749	17.881	20.136	22.380	27.936	20.709	22.110	24.902	27.681	34.564	41.363	25.109	26.809	30.199	33.572	41.934	50.197
	Wind Uplift	12.735	14.341	15.988	19.365	22.860	31.257	13.884	15.673	17.506	21.256	25.128	34.466	16.967	18.991	23.127	27.388	37.702	47.354	18.195	20.416	24.948	29.606	40.927	51.573
8.00	Gravity Load	14.924	16.805	18.736	22.694	26.790	36.629	16.271	18.367	20.514	24.909	29.447	40.390	19.883	22.255	27.102	32.095	44.182	55.493	21.323	23.925	29.236	34.695	47.962	60.437
	δ = Span/180	10.929	11.723	12.515	14.091	15.658	19.537	13.723	14.721	15.716	17.698	19.670	24.553	18.202	19.433	21.887	24.329	30.378	36.355	22.068	23.563	26.542	29.506	36.856	44.119
	Wind Uplift	11.939	13.444	14.989	18.155	21.432	29.303	13.016	14.694	16.412	19.927	23.558	32.312	15.906	17.804	21.681	25.676	35.346	44.394	17.058	19.140	23.389	27.756	38.369	48.350
8.50	Gravity Load		15.817	17.634	21.359	25.214	34.474	15.313	17.287	19.308	23.444	27.715	38.014	18.713	20.946	25.508	30.207	41.584	52.229	20.068	22.518	27.516	32.654	45.140	56.882
	δ = Span/180		10.385	11.086	12.482	13.870	17.306	12.156	13.040	13.921	15.677	17.424	21.749	16.123	17.214	19.388	21.551	26.910	32.203	19.549	20.872	23.511	26.137	32.648	39.081
	Wind Uplift		12.654	14.107	17.087	20.171	27.579	12.251	13.829	15.446	18.755	22.172	30.411	14.971	16.757	20.406	24.166	33.267	41.783	16.055	18.014	22.013	26.123	36.112	45.506
9.00	Gravity Load				20.172	23.813	32.559	14.463	16.326	18.235	22.141	26.175	35.902	17.674	19.782	24.091	28.529	39.273	49.327	18.953	21.267	25.987	30.840	42.633	53.722
	δ = Span/180				11.133	12.372	15.437	10.843	11.631	12.417	13.983	15.541	19.400	14.382	15.354	17.293	19.223	24.003	28.725	17.437	18.618	20.971	23.314	29.121	34.859
	Wind Uplift				16.138	19.050	26.047	11.570	13.061	14.588	17.713	20.940	28.722	14.139	15.826	19.272	22.823	31.419	39.462	15.163	17.013	20.790	24.672	34.106	42.978
9.50	Gravity Load										20.976	24.797	34.012	16.744	18.741	22.823	27.028	37.206	46.731	17.956	20.147	24.619	29.217	40.389	50.894
	δ = Span/180										12.550	13.949	17.411	12.908	13.781	15.521	17.253	21.543	25.780	15.650	16.709	18.822	20.924	26.136	31.286
	Wind Uplift										16.781	19.838	27.210	13.395	14.993	18.258	21.622	29.765	37.385	14.365	16.118	19.696	23.373	32.311	40.716
10.00	Gravity Load										19.927	23.558	32.312	15.906	17.804	21.681	25.676	35.346	44.394	17.058	19.140	23.389	27.756	38.369	48.350
	δ = Span/180										11.327	12.589	15.714	11.649	12.437	14.008	15.570	19.442	23.267	14.124	15.080	16.987	18.884	23.588	28.236
	Wind Uplift										15.942	18.846	25.849	12.725	14.243	17.345	20.541	28.277	35.516	13.646	15.312	18.711	22.205	30.696	38.680

Zeta 2 Ultimate Load Capacities

Zeta 2 Heavy End Bay (Purlin & Rail) - Ultimate Load Capacities in kN

Span (m)	Section No.	2/22514 .18	2/22515 .18	2/22516 .20	2/22518 .25	2/24514 .18	2/24515 .18	2/24516 .20	2/24518 .25	2/26515 .18	2/26516 .20	2/26518 .25	2/26520 .25	2/28515 .18	2/28516 .20	2/28518 .25	2/28520 .25
	Swt. (End Bay)	5.73	5.73	6.37	7.94	6.02	6.02	6.68	8.31	6.30	6.99	8.72	8.72	6.58	7.31	9.11	9.11
6.00	Gravity Load	26.685	30.049	33.501	40.578	29.093	32.842	36.681	44.539	35.525	39.764	48.423	57.345	38.085	42.733	52.218	61.969
	δ = Span/180	25.050	25.050	27.836	34.733	31.463	31.463	34.968	43.649	38.910	43.251	54.006	54.006	47.185	52.456	65.522	65.522
	Wind Uplift	21.348	24.040	26.801	32.462	23.274	26.273	29.345	35.632	28.420	31.811	38.739	45.876	30.468	34.186	41.775	49.575
6.50	Gravity Load	24.633	27.738	30.924	37.457	26.855	30.315	33.860	41.113	32.793	36.705	44.698	52.934	35.155	39.446	48.202	57.202
	δ = Span/180	21.345	21.345	23.718	29.595	26.809	26.809	29.795	37.192	33.154	36.853	46.017	46.017	40.205	44.696	55.830	55.830
	Wind Uplift	19.706	22.190	24.739	29.965	21.484	24.252	27.088	32.891	26.234	29.364	35.759	42.347	28.124	31.557	38.561	45.762
7.00	Gravity Load	22.873	25.757	28.715	34.781	24.937	28.150	31.441	38.177	30.450	34.083	41.506	49.153	32.644	36.628	44.759	53.116
	δ = Span/180	18.404	18.404	20.451	25.518	23.116	23.116	25.691	32.069	28.587	31.776	39.678	39.678	34.667	38.539	48.139	48.139
	Wind Uplift	18.299	20.605	22.972	27.825	19.949	22.520	25.153	30.541	24.360	27.267	33.205	39.322	26.115	29.303	35.807	42.493
7.50	Gravity Load	21.348	24.040	26.801	32.462	23.274	26.273	29.345	35.632	28.420	31.811	38.739	45.876	30.468	34.186	41.775	49.575
	δ = Span/180	16.032	16.032	17.815	22.229	20.136	20.136	22.380	27.936	24.902	27.681	34.564	34.564	30.199	33.572	41.934	41.934
	Wind Uplift	17.079	19.232	21.441	25.970	18.619	21.019	23.476	28.505	22.736	25.449	30.991	36.701	24.374	27.349	33.420	39.660
8.00	Gravity Load	20.014	22.537	25.126	30.433	21.820	24.631	27.511	33.405	26.644	29.823	36.317	43.009	28.564	32.050	39.164	46.477
	δ = Span/180	14.091	14.091	15.658	19.537	17.698	17.698	19.670	24.553	21.887	24.329	30.378	30.378	26.542	29.506	36.856	36.856
	Wind Uplift	16.011	18.030	20.101	24.347	17.456	19.705	22.009	26.724	21.315	23.858	29.054	34.407	22.851	25.640	31.331	37.181
8.50	Gravity Load	18.837	21.211	23.648	28.643	20.536	23.182	25.893	31.440	25.077	28.069	34.181	40.479	26.883	30.164	36.860	43.743
	δ = Span/180	12.482	12.482	13.870	17.306	15.677	15.677	17.424	21.749	19.388	21.551	26.910	26.910	23.511	26.137	32.648	32.648
	Wind Uplift	15.069	16.969	18.918	22.915	16.429	18.546	20.714	25.152	20.061	22.455	27.345	32.383	21.507	24.132	29.488	34.994
9.00	Gravity Load	17.790	20.033	22.334	27.052	19.395	21.894	24.454	29.693	23.684	26.509	32.282	38.230	25.390	28.489	34.812	41.313
	δ = Span/180	11.133	11.133	12.372	15.437	13.983	13.983	15.541	19.400	17.293	19.223	24.003	24.003	20.971	23.314	29.121	29.121
	Wind Uplift	14.232	16.026	17.867	21.642	15.516	17.516	19.563	23.754	18.947	21.207	25.826	30.584	20.312	22.791	27.850	33.050
9.50	Gravity Load					18.374	20.742	23.167	28.130	22.437	25.114	30.583	36.218	24.054	26.989	32.980	39.138
	δ = Span/180					12.550	12.550	13.949	17.411	15.521	17.253	21.543	21.543	18.822	20.924	26.136	26.136
	Wind Uplift					14.700	16.594	18.534	22.504	17.950	20.091	24.466	28.974	19.243	21.591	26.384	31.311
10.00	Gravity Load									21.315	23.858	29.054	34.407	22.851	25.640	31.331	37.181
	δ = Span/180									14.008	15.570	19.442	19.442	16.987	18.884	23.588	23.588
	Wind Uplift									17.052	19.087	23.243	27.526	18.281	20.512	25.065	29.745
10.50	Gravity Load									20.300	22.722	27.670	32.769	21.763	24.419	29.839	35.411
	δ = Span/180									12.705	14.123	17.635	17.635	15.407	17.128	21.395	21.395
	Wind Uplift									16.240	18.178	22.136	26.215	17.410	19.535	23.871	28.329
11.00	Gravity Load									19.377	21.689	26.413	31.279	20.774	23.309	28.483	33.801
	δ = Span/180									11.577	12.868	16.068	16.068	14.039	15.607	19.494	19.494
	Wind Uplift									15.502	17.351	21.130	25.023	16.619	18.647	22.786	27.041

Zeta 2 Ultimate Load Capacities

DESIGNER'S STRUCTURAL PRODUCTS GUIDE

Ultimate Load Capacities

Zeta 2 Butted (Purlin & Rail) - Ultimate Load Capacities in kN

Span (m)	Section No.	2/22514	2/22515	2/22516	2/22518	2/22520	2/22525	2/24514	2/24515	2/24516	2/24518	2/24520	2/24525	2/26515	2/26516	2/26518	2/26520	2/26525	2/26530	2/28515	2/28516	2/28518	2/28520	2/28525	2/28530
	Self Weight	4.47	4.79	5.10	5.73	6.37	7.94	4.69	5.02	5.35	6.02	6.68	8.31	5.26	5.61	6.30	6.99	8.72	10.44	5.49	5.86	6.58	7.31	9.11	10.91
6.00	Gravity Load	13.598	15.312	17.071	20.677	24.408	33.373	14.824	16.735	18.691	22.695	26.830	36.800	18.102	20.262	24.674	29.220	40.225	50.522	19.406	21.775	26.608	31.576	43.651	55.005
	δ = Span/180	10.151	10.889	11.624	13.087	14.543	18.146	12.408	13.311	14.210	16.002	17.785	22.200	16.033	17.118	19.279	21.430	26.759	32.023	19.070	20.362	22.936	25.498	31.849	38.125
	Wind Uplift	10.878	12.249	13.657	16.541	19.527	26.699	11.860	13.388	14.953	18.156	21.464	29.440	14.482	16.209	19.739	23.376	32.180	40.418	15.525	17.420	21.286	25.261	34.921	44.004
6.50	Gravity Load	12.552	14.134	15.758	19.086	22.531	30.806	13.684	15.447	17.253	20.949	24.766	33.969	16.710	18.703	22.776	26.973	37.131	46.636	17.913	20.100	24.561	29.148	40.293	50.774
	δ = Span/180	8.649	9.278	9.904	11.151	12.392	15.462	10.573	11.342	12.108	13.635	15.154	18.916	13.661	14.586	16.427	18.260	22.801	27.286	16.249	17.349	19.543	21.726	27.137	32.485
	Wind Uplift	10.041	11.307	12.606	15.269	18.025	24.645	10.947	12.358	13.803	16.760	19.813	27.175	13.368	14.963	18.221	21.578	29.705	37.309	14.331	16.080	19.649	23.318	32.235	40.619
7.00	Gravity Load	11.655	13.124	14.632	17.723	20.922	28.606	12.707	14.344	16.021	19.453	22.997	31.543	15.516	17.367	21.149	25.046	34.479	43.305	16.634	18.664	22.807	27.066	37.415	47.147
	δ = Span/180	7.458	8.000	8.540	9.615	10.685	13.332	9.116	9.779	10.440	11.757	13.067	16.311	11.779	12.576	14.164	15.745	19.660	23.527	14.011	14.959	16.851	18.733	23.399	28.010
	Wind Uplift	9.324	10.500	11.706	14.178	16.737	22.885	10.165	11.475	12.817	15.562	18.397	25.234	12.413	13.894	16.919	20.037	27.583	34.644	13.307	14.931	18.246	21.652	29.932	37.718
7.50	Gravity Load	10.878	12.249	13.657	16.541	19.527	26.699	11.860	13.388	14.953	18.156	21.464	29.440	14.482	16.209	19.739	23.376	32.180	40.418	15.525	17.420	21.286	25.261	34.921	44.004
	δ = Span/180	6.497	6.969	7.439	8.376	9.308	11.614	7.941	8.519	9.095	10.241	11.382	14.208	10.261	10.955	12.339	13.715	17.126	20.495	12.205	13.031	14.679	16.318	20.383	24.400
	Wind Uplift	8.703	9.800	10.925	13.233	15.621	21.359	9.488	10.710	11.962	14.525	17.171	23.552	11.585	12.968	15.792	18.701	25.744	32.334	12.420	13.936	17.029	20.209	27.937	35.203
8.00	Gravity Load	10.198	11.484	12.803	15.507	18.306	25.030	11.118	12.551	14.018	17.021	20.122	27.600	13.577	15.196	18.506	21.915	30.169	37.892	14.555	16.331	19.956	23.682	32.738	41.254
	δ = Span/180	5.710	6.125	6.538	7.362	8.180	10.207	6.980	7.487	7.993	9.001	10.004	12.488	9.019	9.629	10.845	12.055	15.052	18.013	10.727	11.453	12.901	14.342	17.915	21.445
	Wind Uplift	8.159	9.187	10.242	12.406	14.645	20.024	8.895	10.041	11.215	13.617	16.098	22.080	10.861	12.157	14.805	17.532	24.135	30.313	11.644	13.065	15.965	18.946	26.191	33.003
8.50	Gravity Load			12.050	14.595	17.230	23.558	10.464	11.813	13.194	16.020	18.939	25.976	12.778	14.302	17.417	20.626	28.394	35.663	13.699	15.370	18.782	22.289	30.813	38.827
	δ = Span/180			5.792	6.521	7.246	9.042	6.183	6.632	7.081	7.973	8.862	11.062	7.989	8.529	9.606	10.678	13.333	15.956	9.502	10.146	11.428	12.705	15.869	18.996
	Wind Uplift			9.640	11.676	13.784	18.846	8.371	9.450	10.555	12.816	15.151	20.781	10.222	11.442	13.934	16.501	22.715	28.530	10.959	12.296	15.026	17.831	24.650	31.062
9.00	Gravity Load					16.272	22.249	9.883	11.156	12.461	15.130	17.886	24.533	12.068	13.508	16.450	19.480	26.817	33.682	12.938	14.516	17.739	21.051	29.101	36.670
	δ = Span/180					6.464	8.065	5.515	5.916	6.316	7.112	7.905	9.867	7.126	7.608	8.569	9.525	11.893	14.233	8.476	9.050	10.194	11.332	14.155	16.944
	Wind Uplift					13.018	17.799	7.906	8.925	9.969	12.104	14.309	19.627	9.654	10.806	13.160	15.584	21.453	26.945	10.350	11.613	14.191	16.841	23.281	29.336
9.50	Gravity Load											16.945	23.242	11.433	12.797	15.584	18.455	25.405	31.909	12.257	13.752	16.805	19.943	27.569	34.740
	δ = Span/180											7.094	8.856	6.395	6.828	7.690	8.548	10.674	12.774	7.607	8.122	9.149	10.171	12.704	15.208
	Wind Uplift											13.556	18.594	9.146	10.238	12.467	14.764	20.324	25.527	9.805	11.002	13.444	15.954	22.055	27.792
10.00	Gravity Load												22.080	10.861	12.157	14.805	17.532	24.135	30.313	11.644	13.065	15.965	18.946	26.191	33.003
	δ = Span/180												7.992	5.772	6.162	6.941	7.715	9.633	11.528	6.865	7.330	8.257	9.179	11.466	13.725
	Wind Uplift												17.664	8.689	9.726	11.844	14.026	19.308	24.251	9.315	10.452	12.772	15.157	20.953	26.403

Zed - Ultimate Load Capacities

ipan (m)	Section No.	Z/12513	z/12514	Z/12515	Z/12517	Z/12520	Z/14013	Z/14014	Z/14015	Z/14017	Z/14020	Z/15513	Z/15514	Z/15515	Z/15517	Z/15520	Z/17013	Z/17014	Z/17015	Z/17017	Z/17020	Z/18513	Z/18514	Z/18515	Z/18517	Z/18520
01	Self Weight	2.51	2.70	2.89	3.27	3.83	2.74	2.95	3.15	3.56	4.16	2.89	3.11	3.33	3.76	4.40	3.04	3.28	3.50	3.96	4.63	3.20	3.44	3.68	4.16	4.87
4.50	Gravity Load	10.914	12.261	13.610	16.691	19.839	12.569	14.159	15.781	19.006	23.499	14.048	15.844	17.671	21.308	26.421	15.507	17.512	19.553	23.613	29.365	16.932	19.154	21.411	25.905	32.317
	δ = Span/180	6.768	7.291	7.812	8.848	10.387	9.193	9.904	10.614	12.027	14.126	11.645	12.550	13.451	15.245	17.913	14.453	15.577	16.698	18.929	22.250	17.633	19.007	20.376	23.104	27.166
	Wind Uplift	8.732	9.809	10.888	13.353	15.871	10.055	11.327	12.625	15.205	18.799	11.239	12.675	14.137	17.047	21.137	12.406	14.010	15.642	18.890	23.492	13.546	15.323	17.129	20.724	25.854
5.00	Gravity Load	9.823	11.035	12.249	15.022	17.855	11.312	12.743	14.203	17.106	21.149	12.643	14.260	15.904	19.178	23.779	13.956	15.761	17.597	21.252	26.428	15.239	17.239	19.270	23.314	29.086
	δ = Span/180	5.482	5.906	6.328	7.167	8.414	7.446	8.023	8.598	9.742	11.442	9.433	10.165	10.895	12.348	14.510	11.707	12.617	13.525	15.332	18.023	14.283	15.395	16.504	18.714	22.004
	Wind Uplift	7.858	8.828	9.799	12.018	14.284	9.049	10.194	11.362	13.684	16.919	10.115	11.408	12.723	15.342	19.023	11.165	12.609	14.078	17.001	21.143	12.191	13.791	15.416	18.651	23.269
5.50	Gravity Load	8.930	10.032	11.135	13.657	16.232	10.283	11.584	12.911	15.550	19.227	11.494	12.963	14.458	17.434	21.617	12.687	14.328	15.998	19.320	24.026	13.854	15.671	17.518	21.195	26.442
	δ = Span/180	4.531	4.881	5.230	5.923	6.953	6.154	6.630	7.106	8.051	9.456	7.796	8.401	9.004	10.205	11.992	9.675	10.427	11.178	12.671	14.895	11.804	12.724	13.640	15.466	18.185
	Wind Uplift	7.144	8.026	8.908	10.925	12.986	8.227	9.268	10.329	12.440	15.381	9.195	10.371	11.567	13.947	17.294	10.150	11.463	12.798	15.456	19.221	11.083	12.537	14.014	16.956	21.153
6.00	Gravity Load	8.186	9.196	10.208	12.519	14.879	9.426	10.619	11.836	14.255	17.624	10.536	11.883	13.253	15.981	19.816	11.630	13.134	14.664	17.710	22.024	12.699	14.365	16.058	19.429	24.238
	δ = Span/180	3.807	4.101	4.394	4.977	5.843	5.171	5.571	5.971	6.765	7.946	6.551	7.059	7.566	8.575	10.076	8.130	8.762	9.392	10.647	12.516	9.919	10.691	11.461	12.996	15.281
	Wind Uplift	6.549	7.357	8.166	10.015	11.904	7.541	8.495	9.468	11.404	14.100	8.429	9.506	10.603	12.785	15.853	9.304	10.507	11.732	14.168	17.619	10.159	11.492	12.846	15.543	19.390
6.50	Gravity Load	7.556	8.489	9.422	11.556	13.735	8.701	9.802	10.925	13.158	16.269	9.726	10.969	12.234	14.752	18.292	10.736	12.124	13.536	16.347	20.329	11.722	13.260	14.823	17.934	22.374
	δ = Span/180	3.244	3.495	3.744	4.241	4.979	4.406	4.747	5.087	5.764	6.770	5.582	6.015	6.447	7.307	8.586	6.927	7.466	8.003	9.072	10.664	8.451	9.110	9.766	11.073	13.020
	Wind Uplift	6.045	6.791	7.538	9.244	10.988	6.961	7.842	8.740	10.526	13.015	7.781	8.775	9.787	11.802	14.633	8.588	9.699	10.829	13.078	16.264	9.378	10.608	11.858	14.347	17.899
7.00	Gravity Load						8.080	9.102	10.145	12.218	15.107	9.031	10.185	11.360	13.698	16.985	9.969	11.258	12.569	15.180	18.877	10.885	12.313	13.764	16.653	20.775
	δ = Span/180						3.799	4.093	4.387	4.970	5.838	4.813	5.186	5.559	6.300	7.403	5.973	6.437	6.901	7.823	9.195	7.287	7.855	8.421	9.548	11.227
	Wind Uplift						6.464	7.282	8.116	9.775	12.085	7.225	8.148	9.088	10.959	13.588	7.975	9.006	10.056	12.144	15.102	8.708	9.851	11.011	13.322	16.620
7.50	Gravity Load																9.304	10.507	11.732	14.168	17.619	10.159	11.492	12.846	15.543	19.390
	δ = Span/180																5.203	5.608	6.011	6.814	8.010	6.348	6.842	7.335	8.317	9.780
	Wind Uplift																7.443	8.406	9.385	11.334	14.095	8.128	9.194	10.277	12.434	15.512

Zed Heavy End Bay Purlins - Ultimate Load Capacities in kN

an (m)	Section No.	z/14013 .17	Z/14014 .20	z/14015 .20	z/15513 .17	Z/15514 .20	Z/15515 .20	z/17013 .17	Z/17014 .20	z/17015 .20	Z/18513 .17	Z/18514 .20	Z/18515 .20	z/20013 .17	z/20014 .20	Z/20015 .20
sp	Swt. (End Bay)	3.56	4.16	4.16	3.76	4.40	4.40	3.96	4.63	4.63	4.16	4.87	4.87	4.36	5.10	5.10
4.50	Gravity Load	16.909	19.048	21.230	18.900	21.315	23.774	20.862	23.560	26.305	22.780	25.768	28.804	24.639	27.923	31.259
	$\delta = \text{Span}/180$	12.027	14.126	14.126	15.245	17.913	17.913	18.929	22.250	22.250	23.104	27.166	27.166	27.792	32.687	32.687
	Wind Uplift	13.527	15.239	16.984	15.120	17.052	19.019	16.690	18.848	21.044	18.224	20.615	23.044	19.711	22.338	25.007
5.00	Gravity Load	15.218	17.143	19.107	17.010	19.184	21.396	18.776	21.204	23.674	20.502	23.192	25.924	22.175	25.130	28.133
	d = Span/180	9.742	11.442	11.442	12.348	14.510	14.510	15.332	18.023	18.023	18.714	22.004	22.004	22.511	26.477	26.477
	Wind Uplift	12.174	13.715	15.286	13.608	15.347	17.117	15.021	16.963	18.939	16.401	18.553	20.739	17.740	20.104	22.506
5.50	Gravity Load	13.835	15.585	17.370	15.463	17.440	19.451	17.069	19.276	21.522	18.638	21.083	23.567	20.159	22.846	25.575
	$\delta = \text{Span}/180$	8.051	9.456	9.456	10.205	11.992	11.992	12.671	14.895	14.895	15.466	18.185	18.185	18.604	21.881	21.881
	Wind Uplift	11.068	12.468	13.896	12.371	13.952	15.561	13.655	15.421	17.218	14.910	16.867	18.854	16.127	18.277	20.460
6.00	Gravity Load	12.682	14.286	15.923	14.175	15.986	17.830	15.646	17.670	19.728	17.085	19.326	21.603	18.479	20.942	23.444
	$\delta = \text{Span}/180$	6.765	7.946	7.946	8.575	10.076	10.076	10.647	12.516	12.516	12.996	15.281	15.281	15.633	18.387	18.387
	Wind Uplift	10.145	11.429	12.738	11.340	12.789	14.264	12.517	14.136	15.783	13.668	15.461	17.283	14.783	16.754	18.755
6.50	Gravity Load	11.706	13.187	14.698	13.084	14.757	16.459	14.443	16.311	18.211	15.770	17.840	19.941	17.058	19.331	21.641
	$\delta = \text{Span}/180$	5.764	6.770	6.770	7.307	8.586	8.586	9.072	10.664	10.664	11.073	13.020	13.020	13.320	15.667	15.667
	Wind Uplift	9.365	10.550	11.758	10.467	11.805	13.167	11.554	13.048	14.569	12.616	14.272	15.953	13.646	15.465	17.313
7.00	Gravity Load	10.870	12.245	13.648	12.150	13.703	15.283	13.411	15.146	16.910	14.644	16.565	18.517	15.839	17.950	20.095
	$\delta = \text{Span}/180$	4.970	5.838	5.838	6.300	7.403	7.403	7.823	9.195	9.195	9.548	11.227	11.227	11.485	13.508	13.508
	Wind Uplift	8.696	9.796	10.918	9.720	10.962	12.226	10.729	12.116	13.528	11.715	13.252	14.814	12.672	14.360	16.076
7.50	Gravity Load				11.340	12.789	14.264	12.517	14.136	15.783	13.668	15.461	17.283	14.783	16.754	18.755
	$\delta = \text{Span}/180$				5.488	6.449	6.449	6.814	8.010	8.010	8.317	9.780	9.780	10.005	11.767	11.767
	Wind Uplift				9.072	10.231	11.411	10.014	11.309	12.626	10.934	12.369	13.826	11.827	13.403	15.004
8.00	Gravity Load							11.735	13.252	14.796	12.814	14.495	16.202	13.860	15.706	17.583
	$\delta = \text{Span}/180$							5.989	7.040	7.040	7.310	8.595	8.595	8.793	10.342	10.342
	Wind Uplift							9.388	10.602	11.837	10.251	11.596	12.962	11.088	12.565	14.066
8.50	Gravity Load										12.060	13.642	15.249	13.044	14.783	16.549
	δ = Span/180										6.475	7.614	7.614	7.789	9.161	9.161
	Wind Uplift										9.648	10.914	12.200	10.435	11.826	13.239

Zed Sleeved Purlins - Ultimate Load Capacities in kN

Zed - Ultimate Load Capacities

Zeta Butted (Purlin & Rail) - Ultimate Load Capacities in kN

an (m)	Section No.	Z/12513	Z/12514	Z/12515	Z/12517	Z/12520	Z/14013	Z/14014	Z/14015	Z/14017	Z/14020	Z/15513	Z/15514	Z/15515	Z/15517	Z/15520	Z/17013	Z/17014	Z/17015	Z/17017	Z/17020	Z/18513	Z/18514	Z/18515	Z/18517	Z/18520
sp	Self Weight	2.51	2.70	2.89	3.27	3.83	2.74	2.95	3.15	3.56	4.16	2.89	3.11	3.33	3.76	4.40	3.04	3.28	3.50	3.96	4.63	3.20	3.44	3.68	4.16	4.87
4.50	Gravity Load	7.482	8.405	9.330	11.442	13.600	8.616	9.706	10.818	13.029	16.109	9.630	10.861	12.114	14.607	18.112	10.630	12.005	13.404	16.187	20.130	11.607	13.130	14.677	17.758	22.154
	δ = Span/180	3.372	3.632	3.892	4.408	5.175	4.579	4.934	5.288	5.991	7.037	5.801	6.252	6.701	7.594	8.924	7.200	7.760	8.318	9.430	11.084	8.784	9.468	10.150	11.509	13.533
	Wind Uplift	5.986	6.724	7.464	9.154	10.880	6.893	7.765	8.654	10.423	12.887	7.704	8.689	9.691	11.686	14.490	8.504	9.604	10.723	12.950	16.104	9.286	10.504	11.742	14.207	17.723
5.00	Gravity Load	6.734	7.565	8.397	10.298	12.240	7.754	8.735	9.736	11.726	14.498	8.667	9.775	10.903	13.147	16.301	9.567	10.804	12.063	14.568	18.117	10.447	11.817	13.210	15.982	19.939
	δ = Span/180	2.731	2.942	3.152	3.570	4.191	3.709	3.997	4.283	4.853	5.700	4.699	5.064	5.427	6.151	7.228	5.832	6.285	6.738	7.638	8.978	7.115	7.669	8.222	9.323	10.962
	Wind Uplift	5.387	6.052	6.718	8.238	9.792	6.204	6.988	7.789	9.381	11.599	6.934	7.820	8.722	10.517	13.041	7.654	8.644	9.651	11.655	14.494	8.357	9.454	10.568	12.786	15.951
5.50	Gravity Load						7.049	7.941	8.851	10.660	13.180	7.879	8.887	9.911	11.951	14.819	8.698	9.822	10.967	13.244	16.470	9.497	10.743	12.009	14.529	18.126
	δ = Span/180						3.066	3.303	3.540	4.011	4.711	3.884	4.185	4.486	5.084	5.974	4.820	5.195	5.568	6.312	7.420	5.880	6.338	6.795	7.705	9.059
	Wind Uplift						5.640	6.353	7.081	8.528	10.544	6.304	7.109	7.929	9.561	11.855	6.958	7.858	8.773	10.595	13.176	7.598	8.594	9.607	11.624	14.501
6.00	Gravity Load						6.462	7.280	8.113	9.772	12.082	7.223	8.146	9.085	10.955	13.584	7.973	9.004	10.053	12.140	15.098	8.706	9.848	11.008	13.319	16.616
	δ = Span/180						2.576	2.775	2.974	3.370	3.958	3.263	3.517	3.769	4.272	5.020	4.050	4.365	4.679	5.304	6.235	4.941	5.326	5.710	6.474	7.612
	Wind Uplift						5.170	5.824	6.491	7.817	9.665	5.778	6.517	7.268	8.764	10.867	6.378	7.203	8.042	9.712	12.078	6.964	7.878	8.806	10.655	13.293
6.50	Gravity Load											6.667	7.519	8.387	10.113	12.539	7.359	8.311	9.279	11.206	13.936	8.036	9.090	10.161	12.294	15.338
	δ = Span/180											2.781	2.996	3.212	3.640	4.277	3.451	3.719	3.987	4.519	5.313	4.210	4.538	4.865	5.516	6.486
	Wind Uplift											5.334	6.015	6.709	8.090	10.031	5.888	6.649	7.424	8.965	11.149	6.429	7.272	8.129	9.835	12.270
7.00	Gravity Load																6.834	7.717	8.617	10.406	12.941	7.462	8.441	9.435	11.416	14.242
	δ = Span/180																2.975	3.207	3.438	3.897	4.581	3.630	3.913	4.195	4.756	5.593
	Wind Uplift																5.467	6.174	6.893	8.325	10.353	5.970	6.753	7.548	9.133	11.394
7.50	Gravity Load																					6.964	7.878	8.806	10.655	13.293
	δ = Span/180																					3.162	3.409	3.654	4.143	4.872
	Wind Uplift																					5.572	6.303	7.045	8.524	10.634

'S390' Material

- The majority of the sections illustrated in this manual are manufactured from 'S390' material. BS EN 10147 is the standard code of practice for the specification of hot dipped zinc coated steel strip for
- cold formed sections. Whilst this code does not explicitly carry a standard
- specification for '\$390' material, an appropriate technical description can be calculated that complies with the principles given in Table 1 of this code. These structural parameters can be defined as follows: Yield strength: ReH = 390 N/mm² guaranteed
 - minimum value Tensile strength: $Rm = 468 \text{ N/mm}^2 \text{ minimum value}$
- (i.e. 1.2 x Yield strength in accordance with the design code BS5950:Part5:1998) Elongation: A80 = 14%
- All other technical delivery conditions for the coating, surface finish, quality and treatment as defined in BS EN 10147 are satisfied. Hence, the material reference is 'S390GD+Z275' zinc coated steel, with a minimum guaranteed yield strength of 390 N/mm². The calculation of structural properties for the sections in this manual are in accordance with BS5950:Part5:1998 using the yield and tensile strengths as defined above. Test data has also been analysed to confirm full compliance with the principles adopted.

Cleats

Cleats manufactured from Grade HR4 material to BS 1449, Section 1.2. Baseplate for bolt-on cleats Grade S275 material.

Eaves Beam Brackets

Manufactured from Grade S275 material, and galvanised to BS 729:1971.

Material Specifications

Nothing in this manual represents a performance warranty with regard to material durability. The Building Designer is responsible for assessing the suitability of the products for their intended application, taking into account local environmental conditions. Since our policy is one of continuing product development, Tegral Metal Forming Ltd reserve the right to revise the information shown without notification.

Quality Assurance

IS EN ISO 9001:2000 NSAI Registered Firm Certificate No. 19. 1206.

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